

## Appendix E Horn Lake, Mississippi, Stop-and-Go GPS Survey

### E-1. General

In geographical areas with minimal obstruction of the sky, such as farming areas, along levees and open roads, stop-and-go GPS surveying can be a very effective and efficient method of establishing 2D and/or 3D project control. Stop-and-go surveying can be used to establish horizontal control for topographic and hydrographic surveys as well as 3D ground control for photogrammetric surveys.

### E-2. Project Description

This example survey was conducted by the Memphis District in the vicinity of Horn Lake, Mississippi, approximately 15 miles south of downtown Memphis, Tennessee. A diagram of the project area is shown in Figure E-1. The project consisted of establishing 3D positions on a total of 23 photo control points.

### E-3. Planning Phase

An initial search was performed to locate existing NGRS and USACE horizontal and vertical control within a 10-km radius of the center of the project area. It is important to note that the marks chosen to be occupied by

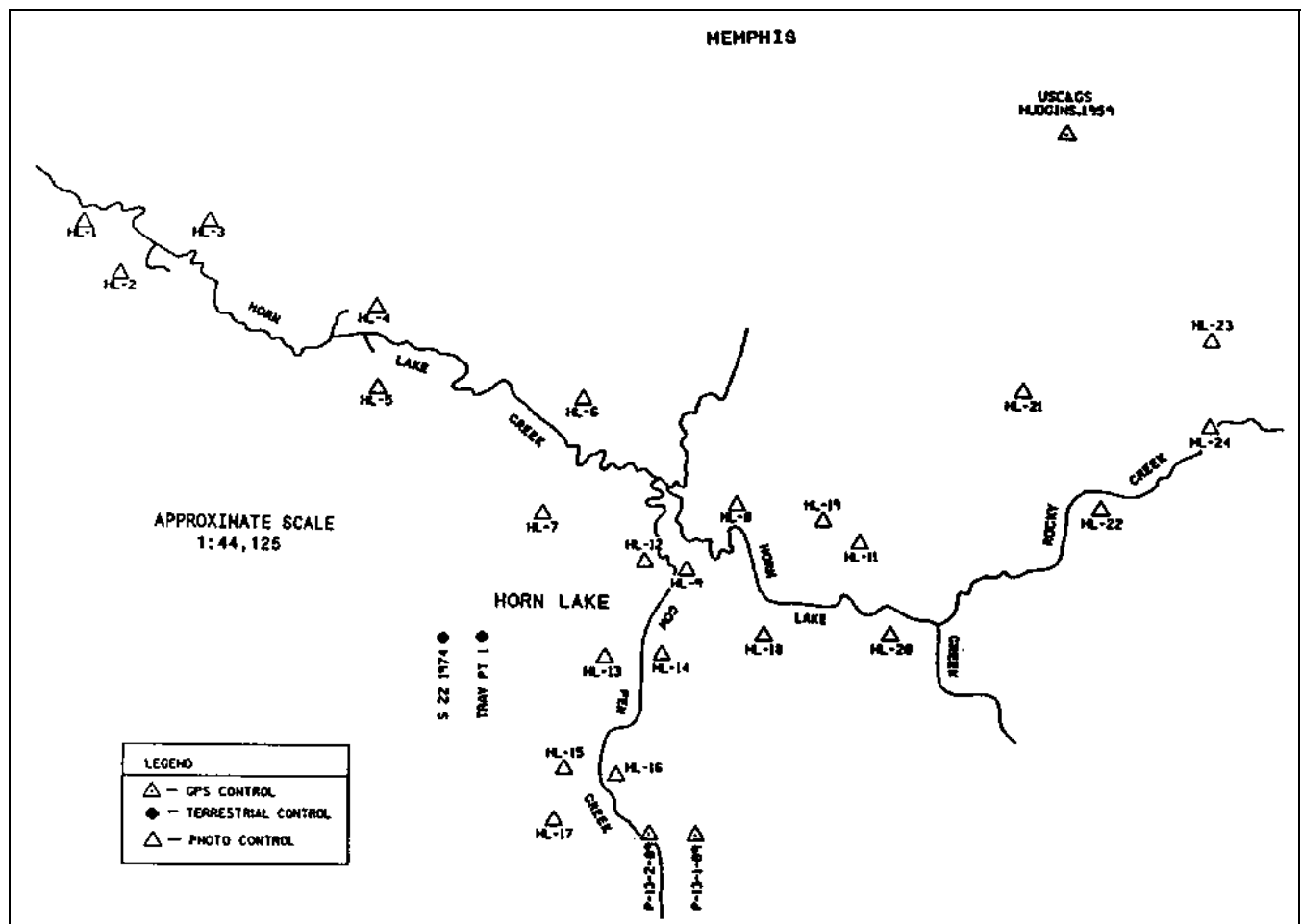


Figure E-1. Horn Lake Project diagram

the reference receiver(s) should be totally unobstructed. A momentary loss of lock (cycle slip) on one or more satellites by the reference receiver(s) could make it impossible to process some or all of the data obtained by the roving receiver(s).

a. Three NGRS horizontal and/or 3D control marks were recovered near the project area; however, due to obstructions at two of the sites, only the 3D mark USC&GS HUDGINS 1959 was suitable as a reference station. Preplanning analysis indicated a minimum of two reference stations were needed to economically obtain the required number of independent baseline measurements to each photo control point. Therefore, to meet this requirement, a pair of intervisible Type A monuments, as defined in EM 1110-1-1002, were installed near the southern limit of the project area with one of the sites (P-13-2-89) being totally free of obstructions greater than 15 degrees above the horizon. Initially, a static GPS survey was performed to establish horizontal positions on the two Type A monuments. A diagram of the initial horizontal control survey is shown in Figure E-2. Refer to the example surveys in Appendix D for details on performing a static horizontal control survey. Figure E-3 shows a partial output of the GEOLAB minimally constrained adjustment of the static baselines observed. A review of the GEOLAB adjustment output reveals the following:

(1) As shown the 2D and 1D station major semi-axis and minor semi-axis are at or less than the few-centimeter level.

(2) The 2D and 1D relative error ellipses between survey points are at or less than the few-centimeter level.

(3) The estimated variance factor in the statistics summary is low (close to a value of 1). Further analysis of the GEOLAB output in Figure E-3 indicates that the adjustment is very acceptable and that the adjusted positions of the two Type A monuments will be more than adequate for use as horizontal control for the stop-and-go survey. Reference stations USC&GS HUDGINS 1959 and P-13-2-89 were chosen because of their unobstructed view of the sky and also their location relative to the project area.

b. One NGRS vertical control mark and three USACE temporary benchmarks (TBM's) were recovered within the project area. The NGRS vertical control mark was 100 percent obstruction free and was thus included within the stop-and-go survey. However, due to obstructions at all three of the TBM's, it was decided not to include these marks within the GPS survey. Instead, differential levels were run from the TBM's to the nearest

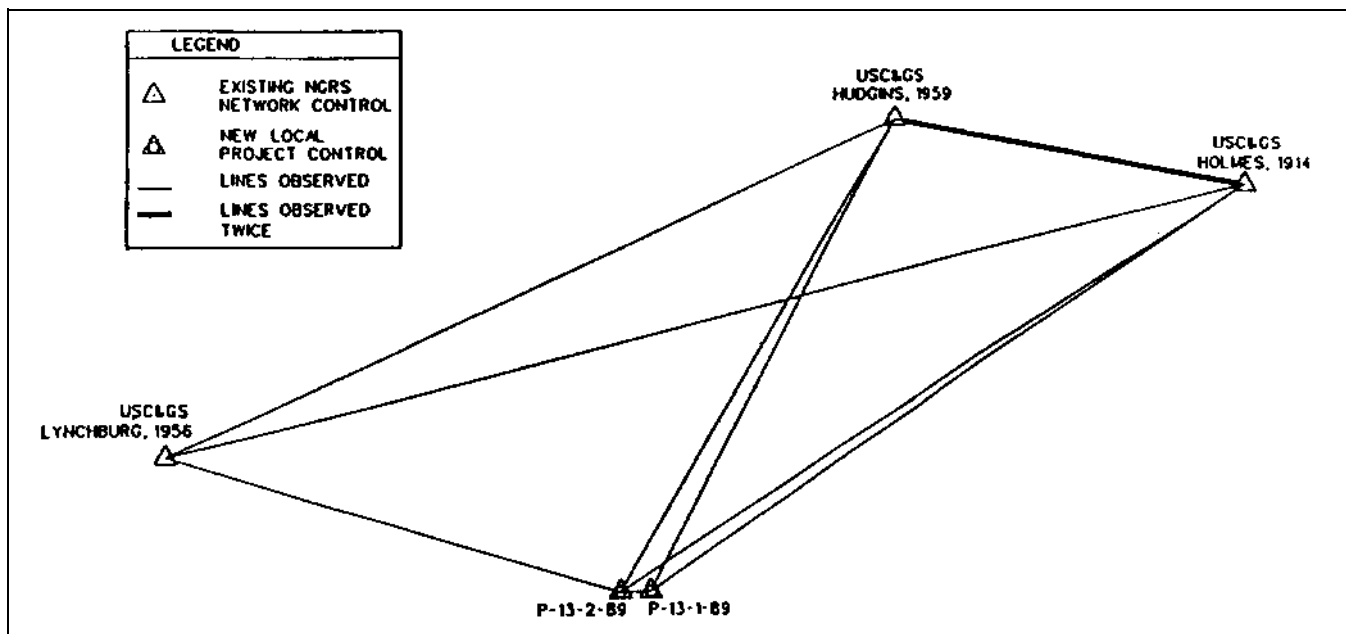


Figure E-2. Initial horizontal control survey

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U.S. ARMY CORPS OF ENGINEERS - MEMPHIS DISTRICT  
GPS #7 - HORN LAKE CREEK AERIAL PHOTO CONTROL  
A= 6378137.000 B= 6356752.314 X0= 0.000 Y0= 0.000 Z0= 0.000  
-----

ELLIPSE:

2-D AND 1-D STATION CONFIDENCE REGIONS ( 95.000 %):

IDENT.	MAJOR SEMI-AXIS	MINOR SEMI-AXIS	AZ(MAJ)	VERTICAL
4098	0.0170	0.0116	119.60	0.0231
3095	0.0154	0.0092	118.54	0.0189
1096	0.0338	0.0136	116.46	0.0333
4097	0.0174	0.0123	118.20	0.0247

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U.S. ARMY CORPS OF ENGINEERS - MEMPHIS DISTRICT  
GPS #7 - HORN LAKE CREEK AERIAL PHOTO CONTROL  
A= 6378137.000 B= 6356752.314 X0= 0.000 Y0= 0.000 Z0= 0.000  
-----

ELLIPSE:

2-D AND 1-D RELATIVE STATION CONFIDENCE REGIONS ( 95.000 %):

FROM	TO	MAJ.SEMI	MIN.SEMI	AZ(MAJ)	VERTICAL	SPATIAL DIST.	PRECISION
3094	1096	0.0338	0.0136	116.46	0.0333	13216.8982	2.559 PPM
3094	4097	0.0174	0.0123	118.20	0.0247	8561.2490	2.029 PPM
3094	4098	0.0170	0.0116	119.60	0.0231	8872.2474	1.912 PPM
3094	3095	0.0154	0.0092	118.54	0.0189	4254.4135	3.612 PPM
4098	1096	0.0309	0.0101	116.01	0.0274	5576.4108	5.539 PPM
4098	4097	0.0055	0.0045	84.37	0.0104	366.6958	14.958 PPM
4098	3095	0.0100	0.0085	135.51	0.0171	6444.1941	1.545 PPM
3095	1096	0.0316	0.0117	116.41	0.0300	9503.6545	3.321 PPM
3095	4097	0.0107	0.0095	125.96	0.0194	6257.9676	1.717 PPM

ELLIPSE successfully completed.  
12:20:35 - Monday, July 10, 1989

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Figure E-3. GEOLAB adjustment output (Continued)

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U.S. ARMY CORPS OF ENGINEERS - MEMPHIS DISTRICT  
GPS #7 - HORN LAKE CREEK AERIAL PHOTO CONTROL  
A= 6378137.000 B= 6356752.314 X0= 0.000 Y0= 0.000 Z0= 0.000  
-----

RESID:

-----  
S T A T I S T I C S      S U M M A R Y  
-----

Residual Critical Value Type	Tau Max
Residual Critical Value	3.1459
Convergence Criterion	0.001000
Final Iteration Counter Value	2
Confidence Level Used	95.0000
Number of Flagged Residuals	0
Estimated Variance Factor	1.3100
Number of Degrees of Freedom	27

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Chi-Square Test on the Variance Factor:

8.1887e-001 < 1.0000 < 2.4271e+000 ?

THE TEST PASSES.

RESID successfully completed.

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Figure E-3. (Concluded)

photo control point. The four points circled in Figure E-4 had an established orthometric height. These heights were used to control the vertical adjustment of the GPS observations. This will be discussed further under the section on adjustments.

c. The aerial photography was completed and photo control points were subsequently selected from observable physical features. The GPS survey party had to locate the points from the photos and monument each on the ground. Obstruction limits at each point were noted during monumentation. After all 23 points were established, further analysis indicated that three of the points, HL-2, HL-5, and HL-7 could not be occupied because of extensive obstructions or no vehicular access. Terrestrial traversing was performed to HL-2 and HL-5 using HL-1 and HL-3 as control and to HL-7 using S 22 1974 and Trav Pt 1 as control. The traverse computations were performed subsequent to the final constrained least squares adjustment of the baselines observed during the stop-and-go survey.

#### E-4. GPS Field Observations

Four SPS (or C/A) code GPS receivers, tracking the carrier phase, were used for the survey. When performing a stop-and-go survey, it is required to maintain lock on at least four satellites. It is recommended that observation-times be scheduled when at least five satellites are visible, so that if lock is lost on one satellite, the survey can continue. All GPS field observations, both static and stop-and-go, were recorded on 28 and 29 June 1989 (days 179 and 180). The reference receiver at HUDGINS was set up to continually record data each day and left unmanned due to its secure location. The reference receiver at P-13-2-89 had one operator monitoring it and there were two personnel per roving receiver: one to drive the vehicle and operate the receiver and the other to position the antenna over the mark. Communication between operators was by two-way radio.

a. The antennas at the reference stations were mounted on a tripod using an optical plummet tribrach with an 8-minute bulls-eye level. The antenna for each roving receiver was mounted on a fixed-height range pole with a 10-minute bulls-eye level supported by a bipod. The antenna, range pole, and bipod were secured to the vehicle by a removable mobile rack shown in Figure E-5. Figure E-6 shows the setup of the antenna, range pole and bipod.

b. A satellite visibility chart was plotted for day 179 and is shown in Figure E-7. Static measurements were

recorded for the first two sessions and the data used to establish horizontal positions on the two Type A monuments. Refer to Figure E-2 for a graphical representation of the baselines observed during sessions 1 and 2. The remainder of the five-satellite window for day 179 was used to record stop-and-go data. The observation schedule developed for day 179 is shown in Figure E-8. The upper portion of the schedule includes the ID number, the name and the position (exact if known, scaled if not) for each station included within the survey. Note that the photo control points were assigned a 5000 series ID. Chapter 8 of this manual discusses recommended conventions for assigning station ID numbers. It recommends a 9000 series number for temporary 3D control. The bottom portion of the schedule includes the date and day of the year in which observations are to be recorded, where each receiver will be for each session, when to start and stop each session (local time), and which satellites to observe.

c. Stop-and-go observations were recorded during the third session on day 179 and during the entire five-satellite window on day 180. During the third session of day 179, both roving receivers were initialized to both of the reference receivers by recording static data for approximately one hour. This method is also referred to as occupying a known baseline, since after post-processing the static data and the data are accepted according to the criteria in this manual, the baseline becomes known. It is important to note that this method requires a sufficient amount of data be collected to ensure that the integer cycle ambiguities are resolved in the baseline solution. Once the integer cycle ambiguity is resolved for a satellite, it remains constant as long as lock is maintained. At the moment lock is lost on a satellite, its integer cycle ambiguity becomes an unknown value and thus again requires resolution. Initially resolving the ambiguities and maintaining lock on at least four satellites is the key to stop-and-go surveying. If lock is not maintained on at least four satellites during a survey, the ambiguities for at least four satellites will need to be resolved again. This can be done during a survey by returning to the last occupied point, which becomes a known baseline after postprocessing. If this method is not practical or desirable, the survey should be stopped at the point of losing lock on at least four satellites and a new survey started. The new survey may be initialized either by recording approximately an hour of static data at a point which has not already been occupied, or by recording a few minutes of data at a point which has been previously occupied. When using the second method, the integer cycle ambiguities for the baselines from both reference receivers to the

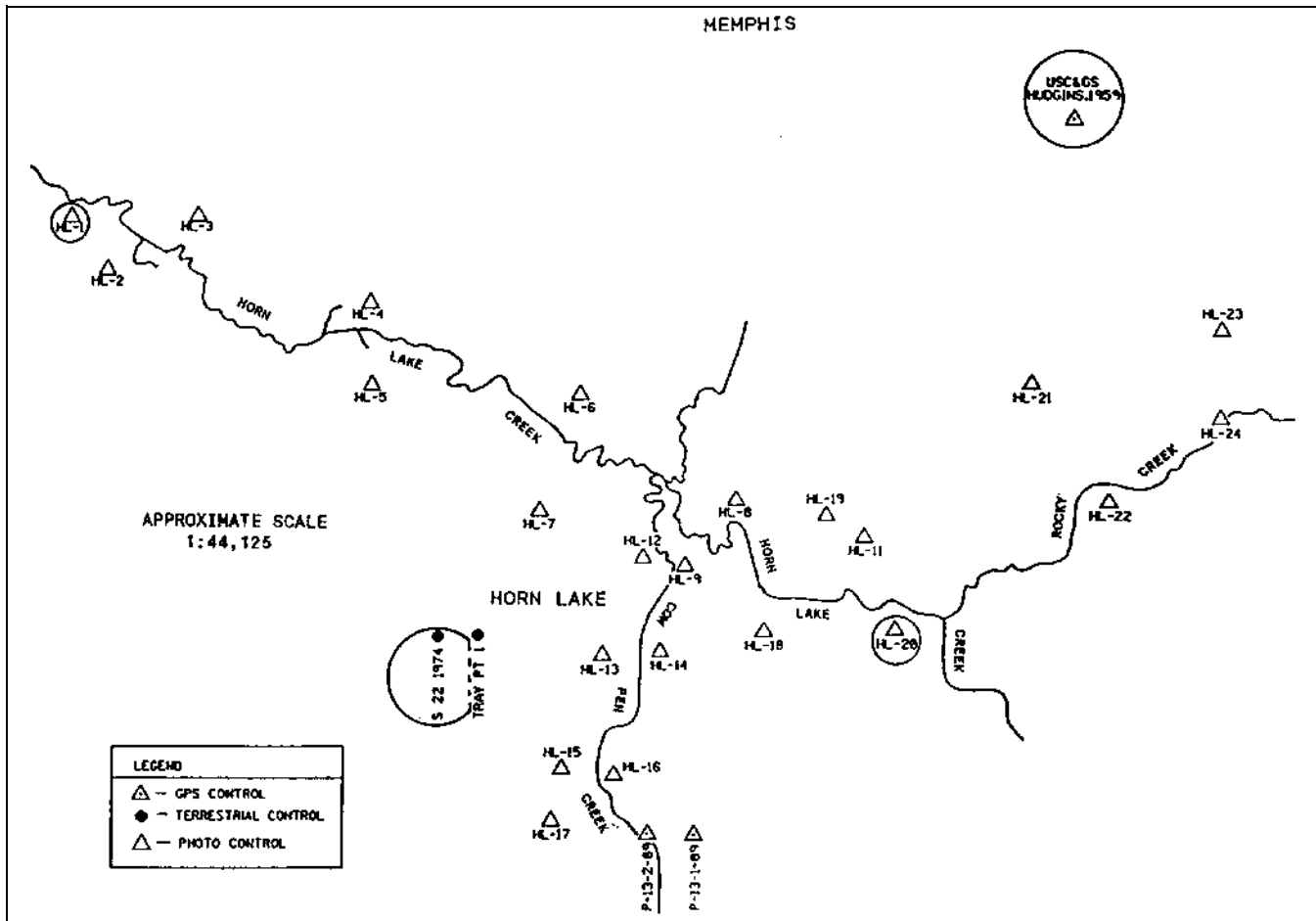


Figure E-4. Vertical project control



Figure E-5. Removable vehicle rack



**Figure E-6. Antenna-range pole-bipod setup**

point must have been resolved. Figures E-9 and E-10 show the baselines for which the integer cycle ambiguities were resolved by collecting static data for approximately one hour by roving receivers 1 and 2, respectively.

*d.* After initializing the stop-and-go survey, the rovers began to travel from point to point, collecting approximately 1.25 minutes of data at each point. This manual recommends data collection of at least 1.5 min at each point. Experience has shown this value should be used as a minimum and, depending on the length of the baselines observed and the accuracy required for the survey, observation times of up to 10 min may be necessary to ensure the desired results are obtained. A data logging sheet such as that shown in Chapter 8 was completed for each reference receiver. For each roving receiver, a stop-and-go field form as shown in Figure E-11 was completed. Figure E-12 shows a field form completed for roving receiver 1 on day 179. The paths taken by each of the rovers on days 179 and 180 can be graphically seen in Figures E-9 and/or E-10. The data collected for the initialization of the survey as well as the data collected at each subsequent point by rover 1 were recorded in one data file. It is recommended that static observations be recorded in a separate file from the stop-and-go data to conform to batch processing methods used in some manufacturer's software. This will be discussed further under the section on post-processing. Referring to Figure E-12, the following is a recommended scheme for recording the data such that a batch processing mode may be utilized.

(1) Start static survey at 22:04 occupying station 4097. Stop static survey at 23:00.

(2) After resetting the antenna over station 4097, start a stop-and-go survey and collect data at 4097 for approximately ten minutes.

(3) Travel to each subsequent point and collect data for at least 1.5 minutes.

(4) Upon losing lock travelling to station 5208, stop the stop-and-go survey.

(5) Continue to 5208, position antenna over mark and start a static survey at approximately 00:42.

(6) Stop the static survey at 01:45, which was very near the end of the four satellite window for day 179.

If additional time had been available in which five satellites were above 15 degrees, another stop-and-go survey could have been started and continued from 5208 to additional points subsequent to the static data collection.

## **E-5. Post-processing Stop-and-Go Data**

At the end of each day's observations, all data were downloaded from the receivers to a portable 386 computer. All processing times quoted in this example are using a 386 computer with a math coprocessor with a 20-MHz clock processing speed. Using a computer with slower clock speeds will significantly increase processing time of all types of GPS data, not only stop-and-go. A review of all field data logging sheets for completeness and correctness was performed after downloading the data. Trimble's Trimvec-Plus survey software was used for all post-processing in this case.

*a.* First, all data collected in the static survey mode were post-processed and the quality of the solutions reviewed. Processed baselines from sessions 1 and 2 of day 179 were used to create a network in GEOLAB and adjusted separately from the photo control. The results of this adjustment have been discussed in paragraph E-3. Since the stop-and-go survey was initialized with known baselines, the dX, dY, and dZ values for the known baselines were required prior to processing the stop-and-go data. These values were obtained from the solution output of the static observations. Figures E-13 and E-14 show the solution summaries for the baselines from each of the reference receivers to roving receiver 1. Notice that the integers were found for each baseline and that the

Table of Azimuth, Elevation and Time for HORN LAKE									
Date : 28 Jun 1989 Time : 15:00 -> 21:00 Cut-off Elevation : 12°									
Latitude : 34° 58' 00" N Longitude : 90° 02' 00" W Zone : - 5:00									
Time	Satellite 3			Satellite 6			Satellite 9		
	AZ	EL		AZ	EL		AZ	EL	
15:00	331°	43°	230°	67°	221°	27°	221°	27°	
15:10	331°	40°	241°	71°	225°	31°	225°	31°	
15:20	331°	53°	257°	74°	229°	35°	229°	35°	
15:30	331°	58°	278°	76°	233°	39°	233°	39°	
15:40	329°	63°	302°	76°	235°	42°	235°	42°	
15:50	325°	68°	322°	74°	237°	46°	237°	46°	
16:00	317°	73°	337°	71°	244°	49°	244°	49°	
16:10	303°	77°	348°	67°	251°	52°	251°	52°	
16:20	277°	79°	357°	64°	258°	54°	258°	54°	
16:30	246°	79°	374°	60°	268°	56°	268°	56°	
16:40	224°	75°	401°	56°	274°	58°	274°	58°	
16:50	212°	71°	438°	52°	283°	59°	283°	59°	
17:00	205°	66°	481°	49°	293°	59°	293°	59°	
17:10	201°	60°	529°	45°	303°	59°	303°	59°	
17:20	199°	55°	581°	42°	313°	59°	313°	59°	
17:30	197°	49°	638°	38°	322°	59°	322°	59°	
17:40	195°	44°	699°	35°	332°	59°	332°	59°	
17:50	194°	38°	764°	32°	341°	56°	341°	56°	
18:00	193°	33°	833°	29°	350°	55°	350°	55°	
18:10	192°	28°	905°	26°	358°	53°	358°	53°	
18:20	191°	23°	980°	23°	366°	50°	366°	50°	
18:30	190°	18°	1058°	20°	374°	47°	374°	47°	
18:40	189°	14°	1139°	17°	382°	44°	382°	44°	
18:50	188°	10°	1223°	14°	390°	41°	390°	41°	
19:00	188°	7°	1310°	11°	398°	38°	398°	38°	
19:10	188°	4°	1400°	8°	406°	35°	406°	35°	
19:20	188°	1°	1493°	5°	414°	32°	414°	32°	
19:30	188°	0°	1589°	2°	422°	29°	422°	29°	
19:40	188°	0°	1688°	0°	430°	26°	430°	26°	
19:50	188°	0°	1789°	0°	438°	23°	438°	23°	
20:00	188°	0°	1892°	0°	446°	20°	446°	20°	
20:10	188°	0°	1997°	0°	454°	17°	454°	17°	
20:20	188°	0°	2104°	0°	462°	14°	462°	14°	
20:30	188°	0°	2213°	0°	470°	11°	470°	11°	
20:40	188°	0°	2324°	0°	478°	8°	478°	8°	
20:50	188°	0°	2437°	0°	486°	5°	486°	5°	
21:00	188°	0°	2552°	0°	494°	2°	494°	2°	

Page 2 of 2 Pages

Page 1 of 2 Pages

Figure E-7. Satellite visibility chart



GPS PROJECT No. 7-K  
HORN LAKE CREEK PHOTO CONTROL Stop and Go SURVEY  
HORN LAKE, MISSISSIPPI

STATION NUMBER	STATION NAME	LATITUDE	LONGITUDE	HEIGHT
3094	HOLMES 1914	34-59-27.00296	89-57-22.44228	123.897
3095	HUDGINS 1959	34-59-52.56136	90-00-07.30092	106.019
1096	LYNCHBERG 1956	34-57-45.46270	90-05-48.68871	94.000
4097	P-13-1-89	34-56-54.00000	90-02-03.00000	91.000
4098	P-13-2-89	34-56-54.00000	90-02-18.00000	84.000
2099	S 22 1974	34-57-45.00000	90-03-18.00000	80.902
5201	HL-1	34-59-30	90-05-05	74.000
5202	HL-2	34-59-18	90-05-00	75.000
5203	HL-3	34-59-30	90-04-30	72.000
5204	HL-4	34-59-13	90-03-38	75.000
5205	HL-5	34-58-40	90-03-38	75.000
5206	HL-6	34-58-45	90-02-35	79.000
5207	HL-7	34-58-15	90-02-45	79.000
5208	HL-8	34-58-20	90-01-47	80.000
5209	HL-9	34-58-02	90-02-02	78.000
5211	HL-11	34-58-10	90-01-08	81.000
5212	HL-12	34-58-05	90-02-17	80.000
5213	HL-13	34-57-40	90-02-28	80.000
5214	HL-14	34-57-38	90-02-20	80.000
5215	HL-15	34-57-12	90-02-40	84.000
5216	HL-16	34-57-10	90-02-22	83.000
5217	HL-17	34-56-58	90-02-43	84.000
5218	HL-18	34-57-45	90-01-40	80.000
5219	HL-19	34-58-15	90-01-20	85.000
5220	HL-20	34-57-45	90-01-00	82.000
5221	HL-21	34-58-47	90-00-19	96.000
5222	HL-22	34-58-20	89-59-55	91.000
5223	HL-23	34-59-02	89-59-13	100.000
5224	HL-24	34-58-39	89-59-13	90.000

WEDNESDAY JUNE 28, 1989 (179)

<u>SESSION 1</u>	<u>SESSION 2</u>	<u>SESSION 3</u>
HUDGINS (3095)	HUDGINS (3095)	HUDGINS (3095)
P-13-2-89(4098)	P-13-2-89(4098)	P-13-2-89(4098)
HOLMES (3094)	HOLMES (3094)	ROVER 2 BEGIN
LYNCHBURG(1096)	P-13-1-89(4097)	ROVER 1 BEGIN
START: 15:40	START: 17:05	START: 18:05
STOP: 16:50	STOP: 18:00	STOP: 20:45
SV's:03,06,08,09, 11,12,13,14	SV's:03,06,08,09 11,12,13,14	SV's:03,06,08,09, 11,12,13,14

Figure E-8. Observation schedule

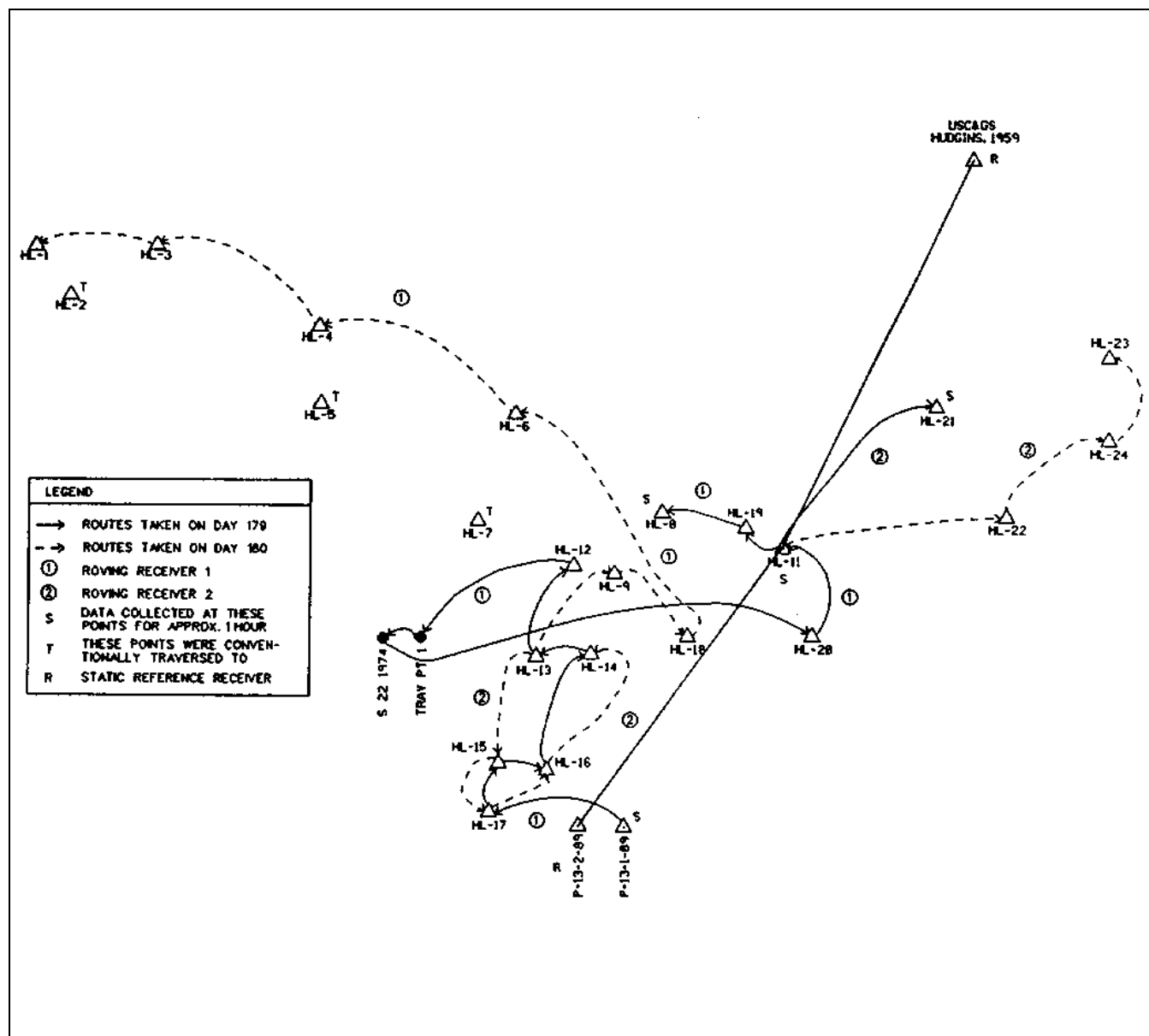


Figure E-9. Observation routes and initialization of rover 1

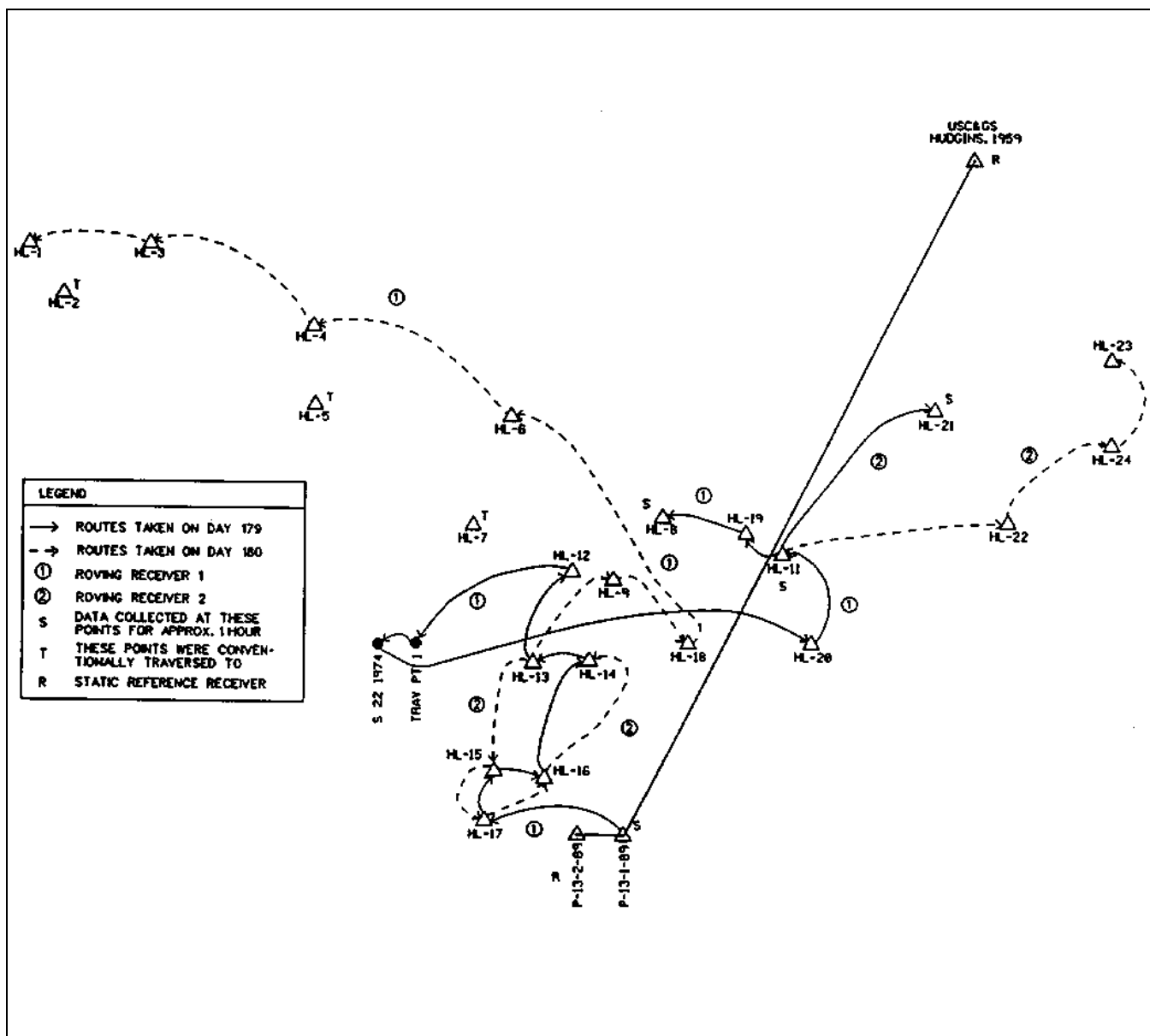


Figure E-10. Observation routes and initialization of rover 2

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PROJECT NAME \_\_\_\_\_ LOCALITY \_\_\_\_\_  
OBSERVER \_\_\_\_\_ AGENCY/FIRM \_\_\_\_\_  
RECEIVER \_\_\_\_\_ S/N \_\_\_\_\_  
ANTENNA \_\_\_\_\_ S/N \_\_\_\_\_  
DATA RECORDING UNIT \_\_\_\_\_ S/N \_\_\_\_\_  
ANTENNA MOUNTING DEVICE \_\_\_\_\_ LAST CALIBRATED: \_\_\_\_\_  
DATE (MM DD YY) \_\_\_\_\_ DAY OF YEAR \_\_\_\_\_

	RECEIVER 1		RECEIVER 2		RECEIVER 3	
STATION NAME						
STATION NUMBER						
UTC TIME OF OBSERVATION	START	STOP	START	STOP	START	STOP

[illegible]

**Figure E-11. Stop-and-go kinematic field form (Continued)**

U.S. ARMY CORPS OF ENGINEERS  
STOP & GO KINEMATIC GPS FIELD FORM

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STOP & GO ROVING RECEIVER CON'T

[illegible]

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ADDITIONAL COMMENTS

[illegible]

**Figure E-11. (Concluded)**

U.S. ARMY CORPS OF ENGINEERS  
STOP & GO KINEMATIC GPS FIELD FORM

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PROJECT NAME HORN LAKE CREEK GPS-7K LOCALITY HORN LAKE, MS  
OBSERVER C. WAITE & C.D. WILBANKS AGENCY/FIRM COE, MEMPHIS DISTRICT  
RECEIVER TRIMBLE 4000 SL S/N 2820A00242  
ANTENNA TRIMBLE MICRO SL S/N 2823A00242  
DATA RECORDING UNIT RECEIVER S/N 2820A00242  
ANTENNA MOUNTING DEVICE RANGE POLE & BIPOD LAST CALIBRATED: 06/27/89  
DATE (MM DD YY) 06/28/89 DAY OF YEAR 179  
\*\*\*\*\*

STATIC REFERENCE RECEIVERS

	RECEIVER 1	RECEIVER 2	RECEIVER 3
STATION NAME	<u>HUDGINS</u>	<u>P-13-2-89</u>	
STATION NUMBER	<u>3095</u>	<u>4098</u>	
UTC TIME OF OBSERVATION	START <u>20:39</u> STOP <u>01:45</u>	START <u>22:03</u> STOP <u>01:46</u>	START STOP

STOP & GO ROVING RECEIVER

STATION NUMBER	STATION NAME	ANTENNA HEIGHT	DATA SET #	COMMENTS
4097	P-13-1-89	1.963	1	STATIC INITIALIZATION 22:04 - 23:00 UTC LOST LOCK TRAVELING TO NEAT PT.
4097	P-13-1-89	"	2	RETURNED TO REINITIALIZE
5217	HL-17	"	3	ANTENNA DISTURBED
5217	HL-17	"	4	REOCCUPIED
5215	HL-15	"	5	
7211	RETURN Pt A	"	6	PK NAIL AT E OR NW END OF MEADOWBROOK RD BRIDGE OVER COW PEN CREEK
5216	HL-16	"	7	
5214	HL-14	"	8	
5213	HL-13	"	9	
5212	HL-12	"	10	
7212	TRAV P-1	"	11	PK NAIL AT N/E COR GOODMAN RD 0.45 M E OF HORN LAKE RD.
2099	5221974	"	12	BENCH MARK ON CULVERT HEADWALL
5220	HL-20	"	13	

Figure E-12. Example stop-and-go kinematic field form (Continued)



1 Aug 96

TRIMVEC GPS RELATIVE POSITIONING SOLUTION SUMMARY: VERSION 89.062

SOLUTION OUTPUT FILE: D:\179\OUT\95971792.fix

STATION 1: Station ID: 3095 Session #: 179-1 Jun 28, 1989 19:22  
 Data-logging start time = 20:39 Data-logging stop time = 01:45

STATION 2: Station ID: 4097 Session #: 179-2 Jun 28, 1989 22:04  
 Data-logging start time = 22:04 Data-logging stop time = 23:00

STATION COORDINATES:

Sta	Ant (m)	Latitude	Longitude	Hgt (m)
1	1.409	34:59'52.56136" N	90:00'07.30092" W	106.019
2 [TRP]	1.963	34:56'53.54740" N	90:02'03.76506" W	91.051
2 [FLT]	1.963	34:56'53.54594" N	90:02'03.76283" W	91.073
2 [FIX]	1.963	34:56'53.54605" N	90:02'03.76258" W	91.089

Origin of station 1 coordinates : User input

SOLUTION SUMMARY:

Solution	dx (m)	dy (m)	dz (m)	dh (m)	RDOP
TRIPLE	-2955.290	-3148.922	-4529.085	-14.969	4.329
FLOAT	-2955.234	-3148.966	-4529.109	-14.946	0.354
*FIXED	-2955.228	-3148.977	-4529.097	-14.930	0.048
FLT-FIX	-0.006	0.011	-0.012	-0.015	

Solution	Slope (m)	sig	Epochs/Rejected	Epoch interval	Epoch increment
TRIPLE	6257.9599	[0.064]	102/ 6	150 (secs)	5 (epochs)
FLOAT	6257.9727	[0.032]	546/ 27	30 (secs)	1 (epochs)
FIXED	6257.9668	[0.014]	545/ 28	30 (secs)	1 (epochs)

Fixed solution quality factor: 10.1  
 Fixed solution rms: 0.039 (cycles)  
 Maximum float - fixed delta: 1.2 (cm)

Integers found, RMS is OK, FIXED solution recommended.

Figure E-13. Static solution summary 3095 -> 4097



TRIMVEC GPS RELATIVE POSITIONING SOLUTION SUMMARY: VERSION 89.062

SOLUTION OUTPUT FILE: D:\179\OUT\98971792.fix

STATION 1: Station ID: 4098 Session #: 179-1 Jun 28, 1989 20:38  
Data-logging start time = 20:42 Data-logging stop time = 01:45

STATION 2: Station ID: 4097 Session #: 179-2 Jun 28, 1989 22:04  
Data-logging start time = 22:04 Data-logging stop time = 23:00

STATION COORDINATES:

Sta	Ant (m)	Latitude	Longitude	Hgt (m)
1	1.590	34:56'53.35126" N	90:02'18.20974" W	83.300
2 [TRP]	1.963	34:56'53.54487" N	90:02'03.76471" W	91.136
2 [FLT]	1.963	34:56'53.54503" N	90:02'03.76453" W	91.114
2 [FIX]	1.963	34:56'53.54458" N	90:02'03.76338" W	91.120

Origin of station 1 coordinates : User input

SOLUTION SUMMARY:

Solution	dx (m)	dy (m)	dz (m)	dh (m)	RDOP
TRIPLE	366.528	-3.238	9.379	7.836	4.303
FLOAT	366.523	-3.217	9.371	7.814	0.354
*FIXED	366.562	-3.230	9.363	7.820	0.047
FLT-FIX	-0.029	0.013	0.008	-0.006	

Solution	Slope (m)	sig	Epochs/Rejected	Epoch interval	Epoch increment
TRIPLE	366.6626	[0.106]	107/ 1	150 (secs)	5 (epochs)
FLOAT	366.6667	[0.006]	570/ 3	30 (secs)	1 (epochs)
FIXED	366.6960	[0.002]	569/ 4	30 (secs)	1 (epochs)

Fixed solution quality factor: 29.7  
Fixed solution rms: 0.024 (cycles)  
Maximum float - fixed delta: 2.9 (cm)

Integers found, RMS is large. (see Trimvec Manual).

Figure E-14. Static solution summary 4098 -> 4097

quality factors were fairly high, meaning that the integer-cycle ambiguities were resolved in both solutions with a high degree of reliability. Even though the RMS was slightly large in comparison to the length of the baseline from 4098 to 4097, the fixed solution quality factor was high and thus the results of the fixed solution are recommended. Had sufficient data not been collected to resolve the integer cycle ambiguities, additional static data collection would be required. An attempt to process stop-and-go data using values from static solutions in which the integers were not found may give erroneous results for the baselines processed. The dX, dY, and dZ values listed for the fixed solution in Figures E-13 and E-14 were used to post-process the stop-and-go data.

b. After all static data were processed, summaries of the stop-and-go sessions were printed. These summaries document the events of the session and the time of their occurrence in GPS seconds of the week. Figure E-15 shows the summary of events that took place during the stop-and-go session on day 179 between the reference receiver at station 3095 and roving receiver initializing on station 4097. The stop-and-go session summary conveys the following details of the survey.

(1) The data file names for the reference and remote (roving) receivers.

(2) The kinematic (stop-and-go) data set number. This number was automatically incremented by one each time a point was occupied.

(3) The station ID of the point being occupied.

(4) The time in seconds that the data set began.

(5) The height of the roving receiver's antenna above the point being occupied.

(6) The time in seconds that the data set ended.

(7) The amount of time in minutes and seconds that data were recorded at the point being occupied.

(8) The last epoch of data recorded before missing satellite PRN 14. The satellite was missing because a cycle slip occurred or a loss of data occurred during download from the receiver to the computer.

(9) The last epoch of data recorded before a cycle slip occurred on satellite PRN 6.

(10) The number of satellites the receiver was locked onto after missing PRN 14 and a cycle slip on PRN 6.

(11) At this point, lock was not maintained on at least four satellites. The survey was reinitialized by returning to the last point occupied; also a new route was chosen to the next point.

(12) The travel time in minutes and seconds that it took to move to the next point, or in this case, to go back to the last occupied point.

(13) The typical observation time at each stop-and-go point.

(14) At this point, lock could not be maintained on at least four satellites. The low elevation of the remaining visible satellites indicated that reinitialization would have been futile.

(15) Since there was slightly over an hour of the four-satellite window remaining on day 179, the survey continued to station 5208 where static observations were recorded for 63 minutes.

(16) Stations within a linked data set were stations consecutively occupied while lock was maintained on at least four satellites. At the point when lock was no longer maintained on at least four satellites, a new linked data set was started.

(17) The station occupation data indicate how many times each point was occupied and to which data set each occupation corresponds.

Figure E-16 shows the summary of events that took place during the stop-and-go session on day 179 between the reference receiver at station 4098 and roving receiver initializing on station 4097. Figures E-15 and E-16 are similar, but they are not exactly the same. A comparison of the two summaries reveals that other than the reference receivers being at different locations, the only other difference is in some of the times when cycle slips or missing satellite data occurred. Since the data from the roving receiver were common to both summaries, the differences in times of missing satellite data were due to obstructions at the reference receiver's location. The roving receiver operator can observe only cycle slips occurring at his receiver. Each receiver in the stop-and-go mode is able to warn the operator with a series of beeps when it no

Trimble Automated Kinematic Processor, Version 89.120  
Kinematic Summary of Session: 179

Reference Receiver Data File: 30951791 (a)  
Remote Receiver Data File: 40971792  
Reference PRN #: 11

KINEMATIC DATA SET: 1 ID: 4097 Time: 338835 Ant height: 2.0000 (b) (c) (d) (e)  
New Scenario Add SV PRN: 12 Time: 339210  
SV Count: 6 Time: 339210  
Last Before Missing PRN: 12 Time: 339360  
SV Count: 5 Time: 339360  
Recovered SV PRN: 12 Time: 339405 \*STATIC INITIALIZATION  
SV Count: 6 Time: 339405 (OR KNOWN BASELINE  
New Scenario Add SV PRN: 14 Time: 340260 AFTER PROCESSING)  
SV Count: 7 Time: 340260  
END OF KINEMATIC DATA ID: 4097 Time: 341985 (f) Ant height: 2.0000  
OBSERVATION TIME: 55:15 (g)

Last Before Missing PRN: 14 Time: 342225 (h)  
Last Before Slip PRN: 6 Time: 342225 (i)  
SV Count: 5 (j) Time: 342225  
Last Before Slip PRN: 8 Time: 342240  
Last Before Slip PRN: 9 Time: 342240  
Last Before Slip PRN: 11 Time: 342240  
Last Before Slip PRN: 12 Time: 342240  
Last Before Slip PRN: 13 Time: 342240  
SV Count: 0 Time: 342240  
After Cycle Slip PRN: 6 Time: 342270 \*LOST LOCK ON SATELLITES  
After Cycle Slip PRN: 8 Time: 342270 WHILE MOVING, RETURNED  
After Cycle Slip PRN: 9 Time: 342270 TO LAST OCCUPIED POINT.  
After Cycle Slip PRN: 11 Time: 342270 (SEE DATA SET 2 BELOW)  
After Cycle Slip PRN: 12 Time: 342270  
Recovered SV PRN: 14 Time: 342270  
SV Count: 6 Time: 342270  
After Cycle Slip PRN: 13 Time: 342315  
SV Count: 7 Time: 342315  
Last Before Slip PRN: 6 Time: 342330  
Last Before Slip PRN: 14 Time: 342330  
SV Count: 5 Time: 342330  
After Cycle Slip PRN: 14 Time: 342375  
SV Count: 6 Time: 342375  
After Cycle Slip PRN: 6 Time: 342420  
Last Before Missing PRN: 8 Time: 342420  
Last Before Missing PRN: 14 Time: 342420  
SV Count: 5 Time: 342420  
Recovered SV PRN: 8 Time: 342465  
SV Count: 6 Time: 342465  
Recovered SV PRN: 14 Time: 342480  
SV Count: 7 Time: 342480  
MOVE TIME: 9:44 (l)

KINEMATIC DATA SET: 2 ID: 4097 Time: 342585 Ant height: 2.0000  
END OF KINEMATIC DATA ID: 4097 Time: 342645 Ant height: 2.0000  
OBSERVATION TIME: 1:15 \*RETURNED TO REINITIALIZE WITH KNOWN BASELINE

(m)

Figure E-15. Kinematic session summary 3095 -> 4097 (Sheet 1 of 6)

1 Aug 96

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      Last Before Slip   PRN:  6   Time:   342810
SV Count:                6         Time:   342810
      After Cycle Slip   PRN:  6   Time:   342840
SV Count:                7         Time:   342840
MOVE TIME:  6:59

KINEMATIC DATA SET:  3   ID: 5217   Time:   343080   Ant height: 2.0000
END OF KINEMATIC DATA   ID: 5217   Time:   343260   Ant height: 2.0000
OBSERVATION TIME:  3:15  *SETUP WAS DISTURBED DURING OCCUPATION,
                        SEE DATA SET 4 FOR REOCCUPATION

MOVE TIME:  0:59

KINEMATIC DATA SET:  4   ID: 5217   Time:   343335   Ant height: 2.0000
END OF KINEMATIC DATA   ID: 5217   Time:   343395   Ant height: 2.0000
OBSERVATION TIME:  1:15

      Last Before Missing PRN:  8   Time:   343545
SV Count:                6         Time:   343545
MOVE TIME:  4: 0

KINEMATIC DATA SET:  5   ID: 5215   Time:   343650   Ant height: 2.0000
KINEMATIC DATA SET:  5   ID: 5215   Time:   343680   Ant height: 2.0000
      Recovered SV       PRN:  8   Time:   343710
SV Count:                7         Time:   343710
END OF KINEMATIC DATA   ID: 5215   Time:   343710   Ant height: 2.0000
OBSERVATION TIME:  1:15

      Last Before Missing PRN:  8   Time:   343845
SV Count:                6         Time:   343845
      Recovered SV       PRN:  8   Time:   343935
      Last Before Missing PRN:  6   Time:   343935
      Recovered SV       PRN:  6   Time:   343965
SV Count:                7         Time:   343965
      Last Before Missing PRN:  8   Time:   343980
SV Count:                6         Time:   343980
      Last Before Missing PRN:  6   Time:   344010
SV Count:                5         Time:   344010
      Recovered SV       PRN:  6   Time:   344130
SV Count:                6         Time:   344130
MOVE TIME:  7:14

KINEMATIC DATA SET:  6   ID: 7211   Time:   344160   Ant height: 2.0000
KINEMATIC DATA SET:  6   ID: 7211   Time:   344220   Ant height: 2.0000
END OF KINEMATIC DATA   ID: 7211   Time:   344220   Ant height: 2.0000
OBSERVATION TIME:  1:15  *THIS POINT WAS ESTABLISHED TO RETURN TO IN CASE
                        LOCK WAS LOST ON THE WAY TO POINT 5216

      Last Before Missing PRN:  6   Time:   344250
SV Count:                5         Time:   344250
      New Scenario Lost SV PRN:  6   Time:   344415
MOVE TIME:  4:14

KINEMATIC DATA SET:  7   ID: 5216   Time:   344490   Ant height: 2.0000
END OF KINEMATIC DATA   ID: 5216   Time:   344550   Ant height: 2.0000
OBSERVATION TIME:  1:15

```

Figure E-15. (Sheet 2 of 6)

Last Before Slip	PRN: 12	Time: 344700	
SV Count: 4		Time: 344700	
Recovered SV	PRN: 8	Time: 344730	
SV Count: 5		Time: 344730	
After Cycle Slip	PRN: 12	Time: 344745	
SV Count: 6		Time: 344745	
Last Before Missing	PRN: 8	Time: 344760	
SV Count: 5		Time: 344760	
MOVE TIME: 6:29			
KINEMATIC DATA SET: 8	ID: 5214	Time: 344955	Ant height: 2.0000
END OF KINEMATIC DATA	ID: 5214	Time: 345015	Ant height: 2.0000
OBSERVATION TIME: 1:15			
Recovered SV	PRN: 8	Time: 345165	
Last Before Slip	PRN: 8	Time: 345165	
After Cycle Slip	PRN: 8	Time: 345210	
SV Count: 6		Time: 345210	
Last Before Slip	PRN: 8	Time: 345270	
SV Count: 5		Time: 345270	
New Scenario Lost SV	PRN: 8	Time: 345330	
MOVE TIME: 5:30			
KINEMATIC DATA SET: 9	ID: 5213	Time: 345360	Ant height: 2.0000
END OF KINEMATIC DATA	ID: 5213	Time: 345420	Ant height: 2.0000
OBSERVATION TIME: 1:14			
MOVE TIME: 6: 0			
KINEMATIC DATA SET: 10	ID: 5212	Time: 345795	Ant height: 2.0000
END OF KINEMATIC DATA	ID: 5212	Time: 345855	Ant height: 2.0000
OBSERVATION TIME: 1:15			
New Scenario Add SV	PRN: 3	Time: 345900	
SV Count: 6		Time: 345900	
Last Before Missing	PRN: 3	Time: 345960	
SV Count: 5		Time: 345960	
Recovered SV	PRN: 3	Time: 346050	
SV Count: 6		Time: 346050	
MOVE TIME: 6:44			
KINEMATIC DATA SET: 11	ID: 7212	Time: 346275	Ant height: 2.0000
END OF KINEMATIC DATA	ID: 7212	Time: 346335	Ant height: 2.0000
OBSERVATION TIME: 1:15			
Last Before Slip	PRN: 3	Time: 346485	
SV Count: 5		Time: 346485	
After Cycle Slip	PRN: 3	Time: 346515	
SV Count: 6		Time: 346515	
MOVE TIME: 3:14			
KINEMATIC DATA SET: 12	ID: 2099	Time: 346545	Ant height: 2.0000
END OF KINEMATIC DATA	ID: 2099	Time: 346620	Ant height: 2.0000
OBSERVATION TIME: 1:30			

Figure E-15. (Sheet 3 of 6)

1 Aug 96

Last Before Slip	PRN: 9	Time: 346815	
SV Count: 5		Time: 346815	
After Cycle Slip	PRN: 9	Time: 346845	
SV Count: 6		Time: 346845	
Last Before Missing	PRN: 9	Time: 346890	
SV Count: 5		Time: 346890	
Recovered SV	PRN: 9	Time: 346935	
Last Before Slip	PRN: 9	Time: 346935	
After Cycle Slip	PRN: 9	Time: 347010	
SV Count: 6		Time: 347010	
MOVE TIME: 8:15			
KINEMATIC DATA SET: 13	ID: 5220	Time: 347130	Ant height: 2.0000
END OF KINEMATIC DATA	ID: 5220	Time: 347190	Ant height: 2.0000
OBSERVATION TIME: 1:15			
MOVE TIME: 4:29			
KINEMATIC DATA SET: 14	ID: 5211	Time: 347475	Ant height: 2.0000
END OF KINEMATIC DATA	ID: 5211	Time: 347535	Ant height: 2.0000
OBSERVATION TIME: 1:15			
Last Before Slip	PRN: 9	Time: 347730	
SV Count: 5		Time: 347730	
After Cycle Slip	PRN: 9	Time: 347760	
SV Count: 6		Time: 347760	
MOVE TIME: 3:45			
KINEMATIC DATA SET: 15	ID: 5219	Time: 347775	Ant height: 2.0000
END OF KINEMATIC DATA	ID: 5219	Time: 347835	Ant height: 2.0000
OBSERVATION TIME: 1:15			
Last Before Missing	PRN: 9	Time: 347895	
Last Before Slip	PRN: 3	Time: 347895	
SV Count: 4		Time: 347895	
After Cycle Slip	PRN: 3	Time: 347940	
Last Before Slip	PRN: 3	Time: 347940	
Last Before Slip	PRN: 11	Time: 347940	
SV Count: 3		Time: 347940	
After Cycle Slip	PRN: 11	Time: 347970	
SV Count: 4		Time: 347970	
Last Before Slip	PRN: 13	Time: 347986	
SV Count: 3		Time: 347986	
Last Before Missing	PRN: 12	Time: 348000	
SV Count: 2		Time: 348000	
After Cycle Slip	PRN: 3	Time: 348015	
After Cycle Slip	PRN: 13	Time: 348015	
Last Before Slip	PRN: 3	Time: 348015	
Last Before Slip	PRN: 11	Time: 348015	
Last Before Slip	PRN: 13	Time: 348015	
Last Before Slip	PRN: 14	Time: 348015	
SV Count: 0		Time: 348015	

⑦ \*AT THIS POINT LOCK WAS  
NOT MAINTAINED ON AT  
LEAST 4 SATELLIES

Figure E-15. (Sheet 4 of 6)

```

    After Cycle Slip PRN: 11 Time: 348045
    Recovered SV PRN: 12 Time: 348045
    After Cycle Slip PRN: 14 Time: 348045
    Last Before Slip PRN: 12 Time: 348045
SV Count: 2 Time: 348045
    After Cycle Slip PRN: 3 Time: 348075
    After Cycle Slip PRN: 12 Time: 348075
    After Cycle Slip PRN: 13 Time: 348075
SV Count: 5 Time: 348075
    Recovered SV PRN: 9 Time: 348090
SV Count: 6 Time: 348090
MOVE TIME: 5: 0

KINEMATIC DATA SET: 16 ID: 5208 ① Time: 348150 Ant height: 2.0000
KINEMATIC DATA SET: 16 ID: 5208 Time: 348195 Ant height: 2.0000
    Last Before Missing PRN: 9 Time: 348420
    SV Count: 5 Time: 348420 *DUE TO THE ABOVE LOSS OF
    New Scenario Lost SV PRN: 9 Time: 348435 LOCK, THIS POINT WAS
    Last Before Missing PRN: 12 Time: 351180 OCCUPIED FOR 63 MINUTES
    SV Count: 4 Time: 351180 AND THE DATA PROCESSED
    New Scenario Lost SV PRN: 12 Time: 351195 AS A STATIC BASELINE.
END OF KINEMATIC DATA ID: 5208 TIME: 351930
OBSERVATION TIME: 63:00

② Linked Data Set # 1: - 4097
  Linked Data Set # 2: - 4097 - 5217 - 5217 - 5215 - 7211 - 5216 - 5214
    - 5213 - 5212 - 7212 - 2099 - 5220 - 5211 - 5219
    Linked Data Set # 3: - 5208

```

Figure E-15. (Sheet 5 of 6)

1 Aug 96

## STATION OCCUPATION INFORMATION



MARK ID: 4097	Occupations:	2
Data Set ID:	1	
Data Set ID:	2	
MARK ID: 5217	Occupations:	2
Data Set ID:	3	
Data Set ID:	4	
MARK ID: 5219	Occupations:	1
Data Set ID:	15	
MARK ID: 5215	Occupations:	1
Data Set ID:	5	
MARK ID: 5216	Occupations:	1
Data Set ID:	7	
MARK ID: 7211	Occupations:	1
Data Set ID:	6	
MARK ID: 5214	Occupations:	1
Data Set ID:	8	
MARK ID: 5213	Occupations:	1
Data Set ID:	9	
MARK ID: 5212	Occupations:	1
Data Set ID:	10	
MARK ID: 7212	Occupations:	1
Data Set ID:	11	
MARK ID: 2099	Occupations:	1
Data Set ID:	12	
MARK ID: 5220	Occupations:	1
Data Set ID:	13	
MARK ID: 5211	Occupations:	1
Data Set ID:	14	
MARK ID: 5208	Occupations:	1
Data Set ID:	16	

Figure E-15. (Sheet 6 of 6)



Trimble Automated Kinematic Processor, Version 89.120  
Kinematic Summary of Session: 179

Reference Receiver Data File: 40981791  
Remote Receiver Data File: 40971792  
Reference PRN #: 11

KINEMATIC DATA SET: 1	ID: 4097	Time: 338835	Ant height: 2.0000
New Scenario Add SV	PRN: 12	Time: 339210	
SV Count: 6		Time: 339210	
Last Before Missing	PRN: 12	Time: 339360	
SV Count: 5		Time: 339360	*STATIC INITIALIZATION
Recovered SV	PRN: 12	Time: 339405	(OR KNOWN BASELINE
SV Count: 6		Time: 339405	AFTER PROCESSING)
New Scenario Add SV	PRN: 14	Time: 340320	
Last Before Missing	PRN: 14	Time: 340320	
Recovered SV	PRN: 14	Time: 340425	
SV Count: 7		Time: 340425	
END OF KINEMATIC DATA	ID: 4097	Time: 341985	Ant height: 2.0000
OBSERVATION TIME: 55:15			

Last Before Missing	PRN: 14	Time: 342225	
Last Before Slip	PRN: 6	Time: 342225	
SV Count: 5		Time: 342225	
Last Before Slip	PRN: 8	Time: 342240	
Last Before Slip	PRN: 9	Time: 342240	
Last Before Slip	PRN: 11	Time: 342240	
Last Before Slip	PRN: 12	Time: 342240	
Last Before Slip	PRN: 13	Time: 342240	
SV Count: 0		Time: 342240	*LOST LOCK ON SATELLITES
After Cycle Slip	PRN: 6	Time: 342270	WHILE MOVING, RETURNED
After Cycle Slip	PRN: 8	Time: 342270	TO LAST OCCUPIED POINT.
After Cycle Slip	PRN: 9	Time: 342270	(SEE DATA SET 2 BELOW)
After Cycle Slip	PRN: 11	Time: 342270	
After Cycle Slip	PRN: 12	Time: 342270	
Recovered SV	PRN: 14	Time: 342270	
SV Count: 6		Time: 342270	
After Cycle Slip	PRN: 13	Time: 342315	
SV Count: 7		Time: 342315	
Last Before Slip	PRN: 6	Time: 342330	
Last Before Slip	PRN: 14	Time: 342330	
SV Count: 5		Time: 342330	
After Cycle Slip	PRN: 14	Time: 342375	
SV Count: 6		Time: 342375	
After Cycle Slip	PRN: 6	Time: 342420	
Last Before Missing	PRN: 8	Time: 342420	
Last Before Missing	PRN: 14	Time: 342420	
SV Count: 5		Time: 342420	
Recovered SV	PRN: 8	Time: 342465	
SV Count: 6		Time: 342465	
Recovered SV	PRN: 14	Time: 342480	
SV Count: 7		Time: 342480	
MOVE TIME: 9:44			

Figure E-16. Kinematic session summary 4098 -> 4097 (Sheet 1 of 6)

1 Aug 96

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KINEMATIC DATA SET: 2   ID: 4097   Time: 342585   Ant height: 2.0000
END OF KINEMATIC DATA   ID: 4097   Time: 342645   Ant height: 2.0000
OBSERVATION TIME: 1:15  *RETURNED TO REINITIALIZE WITH KOWN BASELINE

      Last Before Slip   PRN: 6   Time: 342810
SV Count: 6              Time: 342810
      After Cycle Slip   PRN: 6   Time: 342840
SV Count: 7              Time: 342840
MOVE TIME: 6:59

KINEMATIC DATA SET: 3   ID: 5217   Time: 343080   Ant height: 2.0000
END OF KINEMATIC DATA   ID: 5217   Time: 343260   Ant height: 2.0000
OBSERVATION TIME: 3:15  *SETUP DISTURBED DURING OCCUPATION,
                        SEE DATA SET 4 FOR REOCCUPATION.
MOVE TIME: 0:59

KINEMATIC DATA SET: 4   ID: 5217   Time: 343335   Ant height: 2.0000
END OF KINEMATIC DATA   ID: 5217   Time: 343395   Ant height: 2.0000
OBSERVATION TIME: 1:15

MOVE TIME: 4: 0

KINEMATIC DATA SET: 5   ID: 5215   Time: 343650   Ant height: 2.0000
KINEMATIC DATA SET: 5   ID: 5215   Time: 343680   Ant height: 2.0000
END OF KINEMATIC DATA   ID: 5215   Time: 343710   Ant height: 2.0000
OBSERVATION TIME: 1:15

      Last Before Missing PRN: 8   Time: 343845
SV Count: 6              Time: 343845
      Recovered SV       PRN: 8   Time: 343935
Last Before Missing PRN: 6   Time: 343935
      Recovered SV       PRN: 6   Time: 343965
SV Count: 7              Time: 343965
Last Before Missing PRN: 6   Time: 344010
SV Count: 6              Time: 344010
Last Before Missing PRN: 8   Time: 344055
SV Count: 5              Time: 344055
      Recovered SV       PRN: 6   Time: 344130
Last Before Missing PRN: 6   Time: 344130
New Scenario Lost SV PRN: 6   Time: 344145
      Recovered SV       PRN: 8   Time: 344145
SV Count: 6              Time: 344145
MOVE TIME: 7:14

KINEMATIC DATA SET: 6   ID: 7211   Time: 344160   Ant height: 2.0000
KINEMATIC DATA SET: 6   ID: 7211   Time: 344220   Ant height: 2.0000
END OF KINEMATIC DATA   ID: 7211   Time: 344220   Ant height: 2.0000
OBSERVATION TIME: 1:15  *THIS POINT WAS ESTABLISHED TO RETURN TO IN CASE
                        LOCK WAS LOST ON THE WAY TO POINT 5216.
      Last Before Slip   PRN: 8   Time: 344295
SV Count: 5              Time: 344295
      After Cycle Slip   PRN: 8   Time: 344340
SV Count: 6              Time: 344340
Last Before Missing PRN: 8   Time: 344370
SV Count: 5              Time: 344370
      Recovered SV       PRN: 8   Time: 344430
SV Count: 6              Time: 344430
MOVE TIME: 4:14

```

Figure E-16. (Sheet 2 of 6)

KINEMATIC DATA SET: 7	ID: 5216	Time:	344490	Ant height: 2.0000
END OF KINEMATIC DATA	ID: 5216	Time:	344550	Ant height: 2.0000
OBSERVATION TIME: 1:15				
Last Before Missing	PRN: 8	Time:	344655	
SV Count: 5		Time:	344655	
Last Before Slip	PRN: 12	Time:	344700	
SV Count: 4		Time:	344700	
Recovered SV	PRN: 8	Time:	344730	
SV Count: 5		Time:	344730	
After Cycle Slip	PRN: 12	Time:	344745	
SV Count: 6		Time:	344745	
Last Before Missing	PRN: 8	Time:	344775	
SV Count: 5		Time:	344775	
Recovered SV	PRN: 8	Time:	344865	
SV Count: 6		Time:	344865	
MOVE TIME: 6:29				
KINEMATIC DATA SET: 8	ID: 5214	Time:	344955	Ant height: 2.0000
END OF KINEMATIC DATA	ID: 5214	Time:	345015	Ant height: 2.0000
OBSERVATION TIME: 1:15				
Last Before Missing	PRN: 8	Time:	345090	
SV Count: 5		Time:	345090	
Recovered SV	PRN: 8	Time:	345165	
Last Before Slip	PRN: 8	Time:	345165	
After Cycle Slip	PRN: 8	Time:	345210	
SV Count: 6		Time:	345210	
Last Before Slip	PRN: 8	Time:	345270	
SV Count: 5		Time:	345270	
After Cycle Slip	PRN: 8	Time:	345345	
SV Count: 6		Time:	345345	
MOVE TIME: 5:30				
KINEMATIC DATA SET: 9	ID: 5213	Time:	345360	Ant height: 2.0000
END OF KINEMATIC DATA	ID: 5213	Time:	345420	Ant height: 2.0000
OBSERVATION TIME: 1:14				
Last Before Missing	PRN: 8	Time:	345480	
SV Count: 5		Time:	345480	
New Scenario Lost SV	PRN: 8	Time:	345495	
MOVE TIME: 6: 0				
KINEMATIC DATA SET: 10	ID: 5212	Time:	345795	Ant height: 2.0000
END OF KINEMATIC DATA	ID: 5212	Time:	345855	Ant height: 2.0000
OBSERVATION TIME: 1:15				
New Scenario Add SV	PRN: 3	Time:	345900	
SV Count: 6		Time:	345900	
MOVE TIME: 6:44				
KINEMATIC DATA SET: 11	ID: 7212	Time:	346275	Ant height: 2.0000
END OF KINEMATIC DATA	ID: 7212	Time:	346335	Ant height: 2.0000
OBSERVATION TIME: 1:15				
Last Before Slip	PRN: 3	Time:	346485	
SV Count: 5		Time:	346485	
After Cycle Slip	PRN: 3	Time:	346515	
SV Count: 6		Time:	346515	
MOVE TIME: 3:14				

Figure E-16. (Sheet 3 of 6)

1 Aug 96

KINEMATIC DATA SET: 12	ID: 2099	Time:	346545	Ant height: 2.0000
END OF KINEMATIC DATA	ID: 2099	Time:	346620	Ant height: 2.0000
OBSERVATION TIME: 1:30				
Last Before Slip	PRN: 9	Time:	346815	
SV Count: 5		Time:	346815	
After Cycle Slip	PRN: 9	Time:	346845	
SV Count: 6		Time:	346845	
Last Before Missing	PRN: 9	Time:	346890	
SV Count: 5		Time:	346890	
Recovered SV	PRN: 9	Time:	346935	
Last Before Slip	PRN: 9	Time:	346935	
After Cycle Slip	PRN: 9	Time:	347010	
SV Count: 6		Time:	347010	
MOVE TIME: 8:15				
KINEMATIC DATA SET: 13	ID: 5220	Time:	347130	Ant height: 2.0000
END OF KINEMATIC DATA	ID: 5220	Time:	347190	Ant height: 2.0000
OBSERVATION TIME: 1:15				
MOVE TIME: 4:29				
KINEMATIC DATA SET: 14	ID: 5211	Time:	347475	Ant height: 2.0000
END OF KINEMATIC DATA	ID: 5211	Time:	347535	Ant height: 2.0000
OBSERVATION TIME: 1:15				
Last Before Slip	PRN: 9	Time:	347730	
SV Count: 5		Time:	347730	
After Cycle Slip	PRN: 9	Time:	347760	
SV Count: 6		Time:	347760	
MOVE TIME: 3:45				
KINEMATIC DATA SET: 15	ID: 5219	Time:	347775	Ant height: 2.0000
END OF KINEMATIC DATA	ID: 5219	Time:	347835	Ant height: 2.0000
OBSERVATION TIME: 1:15				
Last Before Missing	PRN: 9	Time:	347895	
Last Before Slip	PRN: 3	Time:	347895	
SV Count: 4		Time:	347895	
After Cycle Slip	PRN: 3	Time:	347940	
Last Before Slip	PRN: 3	Time:	347940	
Last Before Slip	PRN: 11	Time:	347940	
SV Count: 3		Time:	347940	
After Cycle Slip	PRN: 11	Time:	347970	
SV Count: 4		Time:	347970	
Last Before Slip	PRN: 13	Time:	347986	
SV Count: 3		Time:	347986	
Last Before Missing	PRN: 12	Time:	348000	
SV Count: 2		Time:	348000	
After Cycle Slip	PRN: 3	Time:	348015	
After Cycle Slip	PRN: 13	Time:	348015	
Last Before Slip	PRN: 3	Time:	348015	
Last Before Slip	PRN: 11	Time:	348015	
Last Before Slip	PRN: 13	Time:	348015	
Last Before Slip	PRN: 14	Time:	348015	
SV Count: 0		Time:	348015	
After Cycle Slip	PRN: 11	Time:	348045	
Recovered SV	PRN: 12	Time:	348045	
After Cycle Slip	PRN: 14	Time:	348045	
Last Before Slip	PRN: 12	Time:	348045	
				*AT THIS POINT LOCK WAS NOT MAINTAINED ON AT LEAST 4 SATELLITES.

Figure E-16. (Sheet 4 of 6)

```

    After Cycle Slip PRN: 11 Time: 348045
    Recovered SV PRN: 12 Time: 348045
    After Cycle Slip PRN: 14 Time: 348045
    Last Before Slip PRN: 12 Time: 348045
SV Count: 2 Time: 348045
    After Cycle Slip PRN: 3 Time: 348075
    After Cycle Slip PRN: 12 Time: 348075
    After Cycle Slip PRN: 13 Time: 348075
SV Count: 5 Time: 348075
    Recovered SV PRN: 9 Time: 348090
SV Count: 6 Time: 348090
MOVE TIME: 5: 0

KINEMATIC DATA SET: 16 ID: 5208 ① Time: 348150 Ant height: 2.0000
KINEMATIC DATA SET: 16 ID: 5208 Time: 348195 Ant height: 2.0000
    Last Before Missing PRN: 9 Time: 348420
    SV Count: 5 Time: 348420 *DUE TO THE ABOVE LOSS OF
    New Scenario Lost SV PRN: 9 Time: 348435 LOCK, THIS POINT WAS
    Last Before Missing PRN: 12 Time: 351180 OCCUPIED FOR 63 MINUTES
    SV Count: 4 Time: 351180 AND THE DATA PROCESSED
    New Scenario Lost SV PRN: 12 Time: 351195 AS A STATIC BASELINE.
END OF KINEMATIC DATA ID: 5208 TIME: 351930
OBSERVATION TIME: 63:00

② Linked Data Set # 1: - 4097
Linked Data Set # 2: - 4097 - 5217 - 5217 - 5215 - 7211 - 5216 - 5214
- 5213 - 5212 - 7212 - 2099 - 5220 - 5211 - 5219
Linked Data Set # 3: - 5208

```

Figure E-16. (Sheet 5 of 6)

1 Aug 96

STATION OCCUPATION INFORMATION		
-----		
MARK ID: 4097	Occupations:	2
Data Set ID:	1	
Data Set ID:	2	
MARK ID: 5217	Occupations:	2
Data Set ID:	3	
Data Set ID:	4	
MARK ID: 5219	Occupations:	1
Data Set ID:	15	
MARK ID: 5215	Occupations:	1
Data Set ID:	5	
MARK ID: 5216	Occupations:	1
Data Set ID:	7	
MARK ID: 7211	Occupations:	1
Data Set ID:	6	
MARK ID: 5214	Occupations:	1
Data Set ID:	8	
MARK ID: 5213	Occupations:	1
Data Set ID:	9	
MARK ID: 5212	Occupations:	1
Data Set ID:	10	
MARK ID: 7212	Occupations:	1
Data Set ID:	11	
MARK ID: 2099	Occupations:	1
Data Set ID:	12	
MARK ID: 5220	Occupations:	1
Data Set ID:	13	
MARK ID: 5211	Occupations:	1
Data Set ID:	14	
MARK ID: 5208	Occupations:	1
Data Set ID:	16	

Figure E-16. (Sheet 6 of 6)

longer has lock on at least four satellites. The receiver CANNOT warn the roving receiver operator to reinitialize when a combination of the data from the rover and the data from the reference receiver no longer has lock on at least the same four satellites. If the survey is not reinitialized, the data collected subsequent to the loss of lock cannot be processed. Therefore, to ensure that all data collected can be processed, it is very important to choose a location for the reference receiver that has very little or no obstructions of the sky greater than 15 deg above the horizon.

c. If the stop-and-go data have to be manually post-processed, the rule of thumb is it takes twice as long to process the data as it does to collect it. In other words, if stop-and-go data were collected for 2 hr (including move time), it will take approximately 4 hr to post-process the data. If two reference receivers were utilized, it will take approximately 8 hr because twice as many baselines will have to be processed. Manual processing is very labor intensive because each cycle slip must be fixed before processing can continue. As stated previously, when a cycle slip occurs on a satellite, the integer cycle ambiguity for that satellite becomes unknown. When lock is recovered on the satellite, its integer cycle ambiguity must be once again resolved. The ambiguity is resolved by fixing the cycle slip. Missing satellite data are treated as a cycle slip and also require fixing during post-processing. Processing time varies with the number of cycle slips that occurred during the session; the fewer the cycle slips, the quicker the processing will proceed. The actual stepwise procedures of manual processing are beyond the scope of this manual and may vary depending on the software being used. The receiver manufacturer should be consulted for available post-processing training.

d. Some software has a batch mode for post-processing stop-and-go data. To save hours in post-processing time, this option should always be used when possible. Before stop-and-go data are collected, a thorough understanding of the batch processing requirements is strongly recommended. The field procedures used may be modified to meet the batch processing requirements.

e. All stop-and-go baselines observed on days 179 and 180 were manually post-processed. Figure E-17 shows a solution file for the baseline from the reference receiver at 3095 to the roving receiver at 4097. The file naming convention is different from that used for static solution files. Using the file name shown in Figure E-17 as an example, the convention for an eight-digit file name with a three-character extension is as follows.

40971795.k01

where

4097 = the ID of the station being occupied by the roving receiver

179 = the day of the year observations were recorded

5 = the last digit of the ID of the station being occupied by the reference receiver

k = part of extension denoting the file as kinematic

01 = part of extension that is incremented by one each time a station is reoccupied by the same rover on the same day

The solution output for stop-and-go baselines is very similar to the double-difference fixed solution for static baselines. Refer to the survey examples in Appendix D for an annotated output of a double-difference fixed solution.

f. Figure E-9 indicates stations HL-11, HL-13, HL-14, HL-15, HL-16, and HL-17 were occupied at different times by both rovers. Station HL-11 was occupied statically by rover 2 and kinematically by rover 1. The other five stations were occupied kinematically by both rovers. Table E-1 shows the repeatability of the values within baseline solutions comparing a static fixed solution to a stop-and-go kinematic solution and also two kinematic solutions processed from data obtained on different days. The comparisons are shown for baselines processed from both reference receivers. Analysis indicates the repeatability between the static and kinematic solutions was generally less than 20 ppm and that between the two kinematic solutions was about the same. The repeatability in the Y component was consistently the worst of the three components. The expected setup error of the range pole and bipod is greater than an optical plummet tribrach and tripod. This will affect the repeatability of two kinematic baselines, especially if the baselines are processed from data collected by different rovers. Relatively speaking, these results exceed in all cases Second-Order Class II precision requirements and in all cases but two, Second-Order Class I requirements.

07/10/89 13:45:42.30

TRIMBLE NAVIGATION, LTD  
585 NORTH MARY AVENUE  
SUNNYVALE, CALIFORNIA 94086  
U.S.A.

PROGRAM TRIMVEC  
GPS RELATIVE POSITIONING SOLUTION  
VERSION 89.062MB

File name: 40971795.k01  
Coordinate system - WGS-84

Type solution: Double difference  
Value of L12: 1  
L1 solution

Start date/time: 1989/ 6/28 20:39:60. day of year 179 tow 333600.  
Stop date/time: 1989/ 6/29 1:45: 0. day of year 180 tow 351900.

Data available:

station: 1  
sat:11 .....  
sat: 8 .....  
sat: 6 .....  
sat: 9 .....  
sat:13 .....  
sat:12 .....  
sat:14 .....  
sat: 3 .....

station: 2  
sat:11 .....  
sat: 8 .....  
sat: 6 .....  
sat: 9 .....  
sat:13 .....  
sat:12 .....  
sat:14 .....  
sat: 3 .....

Broadcast ephemeris file used: D:\179\30951791.eph

SATELLITE	IODE	HEALTH	WEEK	NO.	TOW(sec)	URA(m)
11	184	0	494	494	349200.00	2.8
8	52	60	494	494	342000.00	32.0
6	27	0	494	494	342030.00	4.0
9	11	0	494	494	345600.00	2.8
13	69	0	494	494	349200.00	2.8
12	194	0	494	494	349200.00	4.0
14	7	0	494	494	349200.00	5.7
3	245	0	494	494	349230.00	2.0

Figure E-17. Kinematic solution output 3095 -> 4097 (Sheet 1 of 4)



```
Broadcast satellite clock correction values
prn      af0      af1      af2      toc
11  -.3036004491D-03  -.5002220860D-11  .0000000000D+00  .3564D+06
8  -.6075738929D-03  -.5923084245D-10  -.2775557562D-16  .3492D+06
6  -.2873353660D-03  -.1796252036D-10  .0000000000D+00  .3492D+06
9  -.3585955128D-03  -.1693933882D-10  -.2775557562D-16  .3528D+06
13  .4193321802D-03  .2160049917D-11  .0000000000D+00  .3564D+06
12  .8332398720D-03  .3751865645D-11  .0000000000D+00  .3564D+06
14  .5646748468D-04  .5911715562D-11  .0000000000D+00  .3564D+06
3  .1517958008D-03  -.1466560207D-10  -.2775557562D-16  .3564D+06
```

No message file for station 1

Origin of station 1 coordinates : User input

STATION (mark) 1 3095

input data file 1 : D:\179\30951791.dat

antenna height(m) 1.409

met values used: pressure(mb) 1013.0  
temperature(deg C) 20.0  
relative humidity(%) 50.0

x (m)	-185.143	lat (dms)	N	34	59	52.56136
y (m)	-5230645.165	elon (dms)	E	269	59	52.69908
z (m)	3637739.935	wlon (dms)	W	90	0	7.30092
		ht (m)				106.0190

No message file for station 2

STATION (mark) 2 4097

input data file 1 : D:\179\40971792.dat

antenna height(m) 1.963

met values used: pressure(mb) 1013.0  
temperature(deg C) 20.0  
relative humidity(%) 50.0

x (m)	-3140.364	lat (dms)	N	34	56	53.54605
y (m)	-5233794.156	elon (dms)	E	269	57	56.23770
z (m)	3633210.847	wlon (dms)	W	90	2	3.76230
		ht (m)				91.1048

Vector 1 originates at station 1 ends at station 2

Vector Standard Deviations (m) :

dx dy dz

Vector 1 .2740151D-02 .6418240D-02 .4906620D-02

Vector correlation matrix :

dx(01) dy(01) dz(01)

dx(01)	1.0000000		
dy(01)	-.7430828	1.0000000	
dz(01)	.6281730	-.8394194	1.0000000

Figure E-17. (Sheet 2 of 4)

1 Aug 96

## STATION 1 TO STATION 2

slope distance (m) 6257.9635 sigma (m) .0022

normal section azimuth (dms) 208 10 42.06

vertical angle (dms) 0 -9 53.01

east(m) north(m) up(m) -2955.109 -5516.259 -17.992

Delta lat(dms) 0 -2 59.01531

Delta lon(dms) 0 -1 56.46138

Delta ht(m) -14.9142

## correlations for baseline 1:

	dx	dy	dz	trop	bias 1	bias 2
	bias 3	bias 4	bias 5	bias 6	bias 7	
dx	1.0000000					
dy	-.7430828	1.0000000				
dz	.6281730	-.8394194	1.0000000			
trop	.0000000	.0000000	.0000000	1.0000000		
bias 1	.0000000	.0000000	.0000000	.0000000	1.0000000	
bias 2	.0000000	.0000000	.0000000	.0000000	.0000000	1.0000000
bias 3	.0000000	.0000000	.0000000	.0000000	.0000000	.0000000
bias 4	.0000000	.0000000	.0000000	.0000000	.0000000	.0000000
bias 5	.0000000	.0000000	.0000000	.0000000	.0000000	.0000000
bias 6	.0000000	.0000000	.0000000	.0000000	.0000000	.0000000
bias 7	.0000000	.0000000	.0000000	.0000000	.0000000	.0000000

	Solution	Sigma
dx (m)	-2955.220	.003
dy (m)	-3148.991	.006
dz (m)	-4529.088	.005
trop (%)	.000	.000
bias 1 (cycle)	9588533.000	.000
bias 2 (cycle)	7281643.000	.000
bias 3 (cycle)	-1603291.000	.000
bias 4 (cycle)	4361315.000	.000
bias 5 (cycle)	7690266.000	.000
bias 6 (cycle)	9590043.000	.000
bias 7 (cycle)	.000	.000
Rdop(norm to 60 sec) is	.295 (m/cycle)	

## All Baseline Vectors:

	dx(m)	dy(m)	dz(m)	dist(m)	dh(m)
From 1 To 2	-2955.220	-3148.991	-4529.088	6257.964	-14.914

Interval between epochs (sec) 15.0

Epoch increment 1

Number of measurements used in solution 30

Number of measurements rejected 0

RMS (cycles) .039

Figure E-17. (Sheet 3 of 4)

```

Elevation mask (deg)      15.0
Edit multiplier           3.5
Modified Hopfield troposphere model used

Best tracking C/A code positions
Station 1
Pdop      2.4
  x (m)    -186.448      lat (dms) N   34   59  52.57129
  y (m)   -5230628.653    elon (dms) E  269   59  52.64760
  z (m)    3637728.748    wlon (dms) W   90   0   7.35241
                                ht (m)      86.0759
clock offset(s)           .50214212D-03
freq offset(s/s)          .70699674D-09

  Code calibration(m)      Carrier calibration(m)
0 - 1      .1860           .0003
0 - 2     -.0625           -.0004
0 - 3     -.3057           -.0005
0 - 4     -.2056           -.0004
0 - 5     -2.9268          .0000
0 - 6     -3.0786          -.0001
0 - 7     -3.0503          -.0005
0 - 8     -3.0967          -.0006
0 - 9     -3.0073          -.0005

Station 2
Pdop      2.5
  x (m)    -3142.263      lat (dms) N   34   56  53.56441
  y (m)   -5233776.319    elon (dms) E  269   57  56.16241
  z (m)    3633199.073    wlon (dms) W   90   2   3.83759
                                ht (m)      69.7407
clock offset(s)           .54571078D-03
freq offset(s/s)          -.13415415D-09

  Code calibration(m)      Carrier calibration(m)
0 - 1      .0161           -.0000
0 - 2     -.0752           -.0004
0 - 3     -.0518           -.0002
0 - 4     -.0840           -.0005
0 - 5     -.1821           -.0001
0 - 6     -.0806           .0002
0 - 7     -.0786           -.0003
0 - 8     -.0518           -.0009
0 - 9     -.1606           -.0009

```

Figure E-17. (Sheet 4 of 4)

**Table E-1**  
**Repeat Baseline Comparison**

File Name	dX	dY	dZ
95111791.FIX	-1564.664	-1813.619	-2636.200
52111795.k01	-1564.666	-1813.590	-2636.214
Diff in meters	0.002	0.029	0.014
Diff in ppm	1.3	16.0	5.3
98111791.FIX	1757.126	1332.128	1902.257
52111798.k01	1757.113	1332.164	1902.226
Diff in meters	0.013	0.036	0.031
Diff in ppm	7.4	27.0	16.3
52131795.k01	-3572.835	-2316.710	-3359.378
52131805.k01	-3572.853	-2316.729	-3359.378
Diff in meters	0.018	0.050	0.029
Diff in ppm	5.0	21.6	8.6
52131795.k01	-251.058	829.025	1179.100
52131808.k01	-251.061	829.020	1179.111
Diff in meters	0.003	0.005	0.011
Diff in ppm	11.9	6.0	9.3

## E-6. Adjustments

GEOLAB adjustment software was used to adjust this example survey. An IOB file was created by adding processed static and kinematic baselines from data obtained on days 179 and 180. Three separate network adjustments were performed using this file.

*a.* The first adjustment was run holding only USC&GS HUDGINS fixed in three dimensions. This adjustment provided a check of the internal precision of the GPS observations. A partial listing of the output for adjustment one is shown in Figure E-18. The data indicated the following:

(1) As shown on page 1 of the GEOLAB adjustment, the number of redundant measurements, or degrees of freedom, within the adjustment was high (87).

(2) As shown on page 31, the 2D and 1D station major semi-axis and minor semi-axis were at or less than the few-centimeter level.

(3) As shown on pages 32 and 33, the 2D and 1D relative error ellipses between survey points were at or less than the few-centimeter level. The precision of all baselines within the adjustment exceeded 30 ppm and 87 percent exceeded 10 ppm.

(4) As shown on page 28, the histogram indicates some of the residuals were higher than anticipated. These higher residuals fell outside of the bell-shaped curve.

(5) As shown on page 29, the estimated variance factor in the statistics summary is somewhat high. Higher residuals and variance factors than seen with static data can be expected when adjusting stop-and-go baselines. Although some high residual values exist in the adjustment, the precision of all baselines relative to their length are within Second-Order Class II requirements and 87 percent are within First-Order requirements. Longer occupation times may help to improve some of the statistical values and baseline precisions if higher orders of accuracy are desired.

*b.* The second adjustment was run holding USC&GS HUDGINS fixed in three dimensions and P-13-2-89 fixed in two dimensions. This adjustment was performed to obtain the final adjusted horizontal positions of all of the photo control points occupied using GPS. After obtaining the adjusted positions from GEOLAB, terrestrial traverse computations were performed to obtain positions on the remaining photo control points. A partial listing of the output for adjustment two is shown in Figure E-19. Here, the statistical values and relative errors increased only slightly compared to adjustment one after holding a second point fixed. The relative precision of all baselines in the adjustment still exceed Second Order Class II requirements.

*c.* The third and final adjustment was ran holding USC&GS HUDGINS fixed in three dimensions and S 22 1974, HL-1 and HL-20 fixed in one dimension. All heights fixed in the adjustment were orthometric or relative to the geoid. If elevations relative to the geoid are desired, a separate vertical adjustment is required holding only one point fixed horizontally. Horizontal and vertical adjustments should not be combined because precisions in the horizontal plane will affect precisions in the vertical plane and vice versa. A partial listing of the output for adjustment three is shown in Figure E-20. Page 33 of the output indicates that the 1D confidence region for each station in the adjustment is less than 0.1 m.

## E-7. Project Summary

Positions in 3D were developed for 23 photo control points as well as 2D positions for two new Type A monuments.

*a.* The field work for the survey was completed in approximately 35 hr with a task breakdown as follows.

(1) 8 hr for presurvey reconnaissance, 2-man crew.

-----  
U.S. ARMY CORPS OF ENGINEERS - MEMPHIS DISTRICT  
HORN LAKE CREEK PHOTO CONTROL - GPS STOP & GO KINEMATIC SURVEY  
A= 6378137.000 B= 6356752.314 X0= 0.000 Y0= 0.000 Z0= 0.000  
-----

PREPARE:

09:34:43 - Tuesday, October 09, 1990

Input from: <GPS7K\_1.iob>

Output to: <GPS7K\_1.LST>

PREPARE successfully completed.

-----  
GeoLab - V1.91S, (C) 1985/86/87/88/89 BitWise Ideas Inc. [103208976] Page 0

-----  
U.S. ARMY CORPS OF ENGINEERS - MEMPHIS DISTRICT  
HORN LAKE CREEK PHOTO CONTROL - GPS STOP & GO KINEMATIC SURVEY  
A= 6378137.000 B= 6356752.314 X0= 0.000 Y0= 0.000 Z0= 0.000  
-----

GETUP:

PARAMETERS		OBSERVATIONS	
Description	Number	Description	Number
All Stations	24	Directions	0
Fixed Stations	1	Distances	0
Free 3-D Stations	23	Azimuths	0
Free 2-D Stations	0	Vertical Angles	0
Free 1-D Stations	0	Zenithal Angles	0
Coord. Parameters	69	Angles	0
Astro. Latitudes	0	Heights	0
Astro. Longitudes	0	Height Differences	0
Geoid Records	0	Auxiliary Params.	0
All Aux. Pars.	0	2-D Coords.	0
Direction Pars.	0	2-D Coord. Diffs.	0
Scale Parameters	0	3-D Coords.	0
Constant Pars.	0	3-D Coord. Diffs.	156
Rotation Pars.	0		
Translation Pars.	0		
Total Parameters		Total Observations	
	69		156
Degrees of Freedom = 87			

-----  
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Figure E-18. Minimally constrained horizontal adjustment (Sheet 1 of 6)

-----  
U.S. ARMY CORPS OF ENGINEERS - MEMPHIS DISTRICT  
HORN LAKE CREEK PHOTO CONTROL - GPS STOP & GO KINEMATIC SURVEY  
A= 6378137.000 B= 6356752.314 X0= 0.000 Y0= 0.000 Z0= 0.000  
-----

RESID:

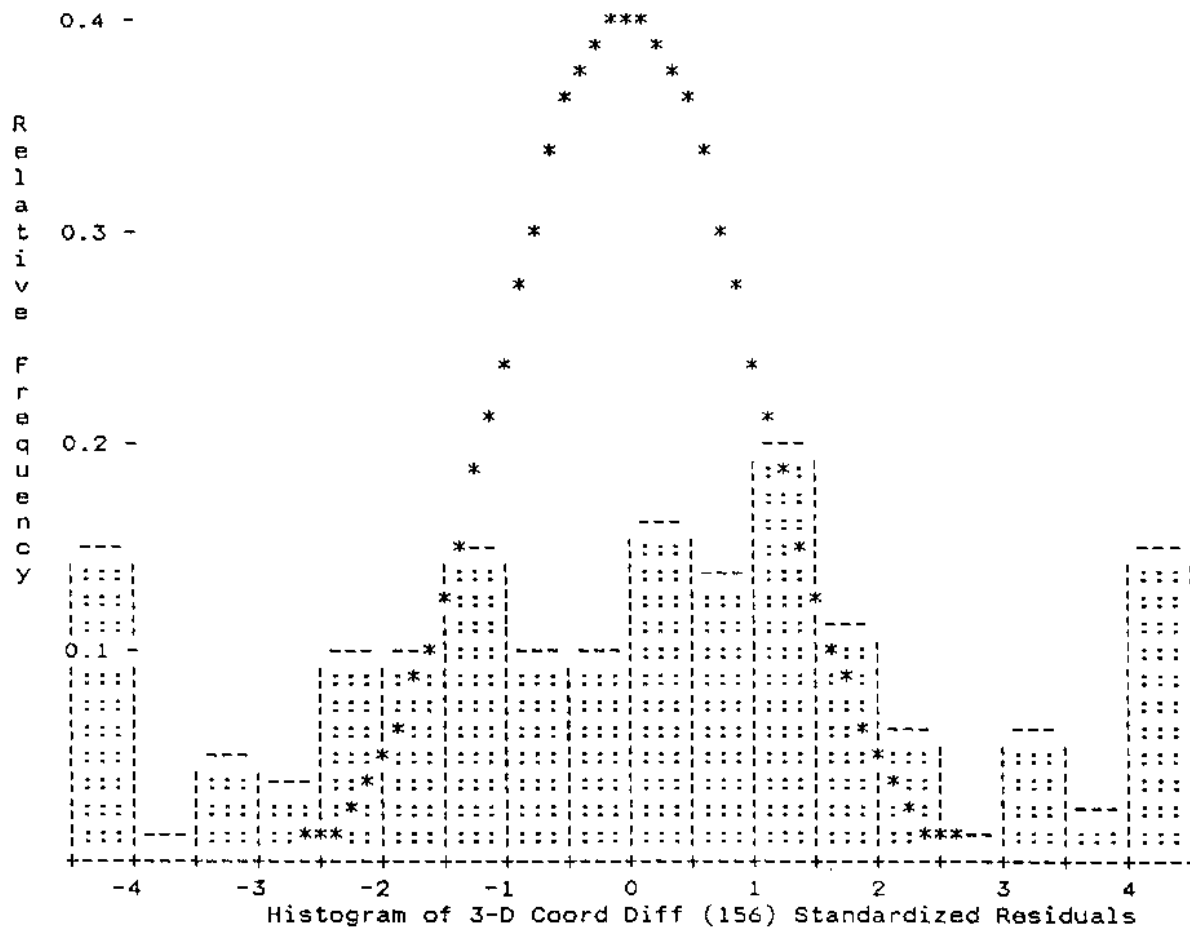


Figure E-18. (Sheet 2 of 6)

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-----
                U.S. ARMY CORPS OF ENGINEERS - MEMPHIS DISTRICT
            HORN LAKE CREEK PHOTO CONTROL - GPS STOP & GO KINEMATIC SURVEY
A= 6378137.000 B= 6356752.314 X0=      0.000 Y0=      0.000 Z0=      0.000
-----

```

RESID:

```

-----
                S T A T I S T I C S      S U M M A R Y
-----

```

Residual Critical Value Type	Tau Max
Residual Critical Value	3.6542
Convergence Criterion	0.001000
Final Iteration Counter Value	2
Confidence Level Used	95.0000
Number of Flagged Residuals	29
Estimated Variance Factor	9.2151
Number of Degrees of Freedom	87

Chi-Square Test on the Variance Factor:

6.9901e+000 < 1.0000 < 1.2708e+001 ?

!!!!!!!!!!!!!!!!!!!!!!!!!!!! THE TEST FAILS !!!!!!!!!!!!!!!!!!!!!!!!!!!!!

RESID successfully completed.

Figure E-18. (Sheet 3 of 6)

1 Aug 96

-----  
 U.S. ARMY CORPS OF ENGINEERS - MEMPHIS DISTRICT  
 HORN LAKE CREEK PHOTO CONTROL - GPS STOP & GO KINEMATIC SURVEY  
 A= 6378137.000 B= 6356752.314 X0= 0.000 Y0= 0.000 Z0= 0.000  
 -----

ELLIPSE:

2-D AND 1-D STATION CONFIDENCE REGIONS ( 95.000 %):

IDENT.	MAJOR SEMI-AXIS	MINOR SEMI-AXIS	AZ(MAJ)	VERTICAL
4098	0.0037	0.0030	145.07	0.0084
4097	0.0111	0.0098	104.55	0.0231
5211	0.0086	0.0079	76.51	0.0167
5221	0.0229	0.0137	4.04	0.0536
5208	0.0353	0.0135	1.88	0.0593
5213	0.0079	0.0059	114.15	0.0117
2099	0.0190	0.0128	23.44	0.0402
5212	0.0227	0.0085	9.19	0.0985
5214	0.0139	0.0078	153.79	0.0406
5215	0.0076	0.0055	160.93	0.0217
5216	0.0056	0.0035	157.21	0.0196
5217	0.0152	0.0107	143.90	0.0344
5219	0.0167	0.0103	11.22	0.0322
5220	0.0143	0.0096	17.16	0.0295
7212	0.0102	0.0073	24.15	0.0219
5222	0.0069	0.0057	87.95	0.0139
5224	0.0150	0.0122	94.27	0.0321
5223	0.0078	0.0063	101.33	0.0176
5209	0.0074	0.0060	66.59	0.0113
5218	0.0123	0.0097	128.12	0.0201
5206	0.0116	0.0086	142.23	0.0263
5201	0.0125	0.0082	28.16	0.0260
5203	0.0115	0.0044	8.95	0.0540

Figure E-18. (Sheet 4 of 6)



1 Aug 96

-----  
 U.S. ARMY CORPS OF ENGINEERS - MEMPHIS DISTRICT  
 HORN LAKE CREEK PHOTO CONTROL - GPS STOP & GO KINEMATIC SURVEY  
 A= 6378137.000 B= 6356752.314 X0= 0.000 Y0= 0.000 Z0= 0.000  
 -----

ELLIPSE:

2-D AND 1-D RELATIVE STATION CONFIDENCE REGIONS ( 95.000 %):

FROM	TO	MAJ.SEMI	MIN.SEMI	AZ(MAJ)	VERTICAL	SPATIAL DIST.	PRECISION
4098	4097	0.0110	0.0097	99.22	0.0227	366.6975	29.965 PPM
4098	5211	0.0082	0.0071	72.51	0.0145	2912.1464	2.824 PPM
4098	5221	0.0231	0.0140	3.76	0.0541	4613.1049	5.003 PPM
4098	5208	0.0353	0.0132	1.93	0.0591	2733.5199	12.901 PPM
4098	2099	0.0188	0.0124	23.86	0.0395	2209.5900	8.528 PPM
4098	5212	0.0226	0.0081	9.40	0.0983	2184.0854	10.334 PPM
4098	5214	0.0140	0.0078	153.61	0.0398	1476.6279	9.461 PPM
4098	5215	0.0079	0.0057	158.56	0.0214	801.5978	9.828 PPM
4098	5216	0.0060	0.0039	154.86	0.0183	542.8320	10.996 PPM
4098	5217	0.0151	0.0106	143.88	0.0340	661.3902	22.792 PPM
4098	5219	0.0168	0.0104	11.02	0.0324	2891.5861	5.799 PPM
4098	5220	0.0139	0.0091	18.11	0.0286	2566.7333	5.434 PPM
4098	7212	0.0097	0.0064	26.24	0.0204	2016.1771	4.824 PPM
4098	5222	0.0070	0.0059	89.23	0.0142	4430.4221	1.574 PPM
4098	5224	0.0150	0.0122	94.28	0.0321	5477.8680	2.742 PPM
4098	5213	0.0076	0.0056	114.08	0.0113	1463.0776	5.162 PPM
4098	5223	0.0080	0.0065	102.72	0.0180	5905.2055	1.354 PPM
4098	5209	0.0078	0.0067	68.72	0.0132	2151.8382	3.631 PPM
4098	5218	0.0119	0.0094	127.14	0.0189	1886.0709	6.326 PPM
4098	5206	0.0118	0.0088	142.32	0.0268	3467.4965	3.412 PPM
4098	5203	0.0114	0.0041	9.23	0.0538	5895.6948	1.932 PPM
4098	5201	0.0127	0.0084	27.81	0.0264	6501.9484	1.947 PPM
4098	3095	0.0037	0.0030	145.07	0.0084	6444.1890	0.580 PPM
4097	3095	0.0111	0.0098	104.55	0.0231	6257.9628	1.776 PPM
3095	5211	0.0086	0.0079	76.51	0.0167	3561.8762	2.428 PPM
3095	5208	0.0353	0.0135	1.88	0.0593	3860.2180	9.155 PPM
3095	5221	0.0229	0.0137	4.04	0.0536	2049.7519	11.170 PPM
3095	5213	0.0079	0.0059	114.15	0.0117	5423.8080	1.456 PPM
3095	2099	0.0190	0.0128	23.44	0.0402	6228.3592	3.056 PPM
3095	5212	0.0227	0.0085	9.19	0.0985	4670.3247	4.871 PPM
3095	5214	0.0139	0.0078	153.79	0.0406	5123.7279	2.718 PPM
3095	5215	0.0076	0.0055	160.93	0.0217	6286.9779	1.201 PPM
3095	5216	0.0056	0.0035	157.21	0.0196	6093.8864	0.915 PPM
3095	5217	0.0152	0.0107	143.90	0.0344	6657.5099	2.288 PPM
3095	5219	0.0167	0.0103	11.22	0.0322	3553.0160	4.696 PPM
3095	5220	0.0143	0.0096	17.16	0.0295	4130.2034	3.452 PPM
3095	7212	0.0102	0.0073	24.15	0.0219	5986.6100	1.705 PPM
3095	5222	0.0069	0.0057	87.95	0.0139	2948.5824	2.328 PPM
3095	5224	0.0150	0.0122	94.27	0.0321	2553.4890	5.881 PPM
3095	5223	0.0078	0.0063	101.33	0.0176	1964.3998	3.985 PPM
3095	5209	0.0074	0.0060	66.59	0.0113	4497.4302	1.649 PPM
3095	5218	0.0123	0.0097	128.12	0.0201	4558.2365	2.702 PPM
3095	5206	0.0116	0.0086	142.23	0.0263	4289.4530	2.703 PPM
3095	5201	0.0125	0.0082	28.16	0.0260	7706.4120	1.628 PPM

-----  
 GeoLab - V1.91S, (C) 1985/86/87/88/89 BitWise Ideas Inc. [103208976] Page 32

Figure E-18. (Sheet 5 of 6)

1 Aug 96

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-----
                U.S. ARMY CORPS OF ENGINEERS - MEMPHIS DISTRICT
            HORN LAKE CREEK PHOTO CONTROL - GPS STOP & GO KINEMATIC SURVEY
A= 6378137.000 B= 6356752.314 X0=      0.000 Y0=      0.000 Z0=      0.000
-----

```

ELLIPSE:

2-D AND 1-D RELATIVE STATION CONFIDENCE REGIONS ( 95.000 %):

```

-----
FROM      TO      MAJ.SEMI MIN.SEMI AZ(MAJ) VERTICAL SPATIAL DIST.    PRECISION
-----
3095      5203      0.0115  0.0044   8.95   0.0540   6734.0519   1.709 PPM
-----

```

ELLIPSE successfully completed.  
09:40:34 - Tuesday, October 09, 1990

Figure E-18. (Sheet 6 of 6)

```

-----
                U.S. ARMY CORPS OF ENGINEERS - MEMPHIS DISTRICT
            HORN LAKE CREEK PHOTO CONTROL - GPS STOP & GO KINEMATIC SURVEY
A= 6378137.000 B= 6356752.314 X0=      0.000 Y0=      0.000 Z0=      0.000
-----

```

PREPARE:

10:00:35 - Tuesday, October 09, 1990

Input from: <GPS7K\_2.iob>

Output to: <GPS7K\_2.LST>

PREPARE successfully completed.

```

-----
GeoLab - V1.91S, (C) 1985/86/87/88/89 Bitwise Ideas Inc. [103208976] Page 0
-----

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-----
                U.S. ARMY CORPS OF ENGINEERS - MEMPHIS DISTRICT
            HORN LAKE CREEK PHOTO CONTROL - GPS STOP & GO KINEMATIC SURVEY
A= 6378137.000 B= 6356752.314 X0=      0.000 Y0=      0.000 Z0=      0.000
-----

```

GETUP:

PARAMETERS		OBSERVATIONS	
Description	Number	Description	Number
All Stations	24	Directions	0
Fixed Stations	1	Distances	0
Free 3-D Stations	23	Azimuths	0
Free 2-D Stations	0	Vertical Angles	0
Free 1-D Stations	0	Zenithal Angles	0
Coord. Parameters	69	Angles	0
Astro. Latitudes	0	Heights	0
Astro. Longitudes	0	Height Differences	0
Geoid Records	0	Auxiliary Params.	0
All Aux. Pars.	0	2-D Coords.	2
Direction Pars.	0	2-D Coord. Diffs.	0
Scale Parameters	0	3-D Coords.	0
Constant Pars.	0	3-D Coord. Diffs.	156
Rotation Pars.	0		
Translation Pars.	0		
	-----		-----
Total Parameters	69	Total Observations	158
-----		-----	
Degrees of Freedom =		89	

```

-----
GeoLab - V1.91S, (C) 1985/86/87/88/89 Bitwise Ideas Inc. [103208976] Page 1
-----

```

Figure E-19. Fully constrained horizontal adjustment (Sheet 1 of 6)

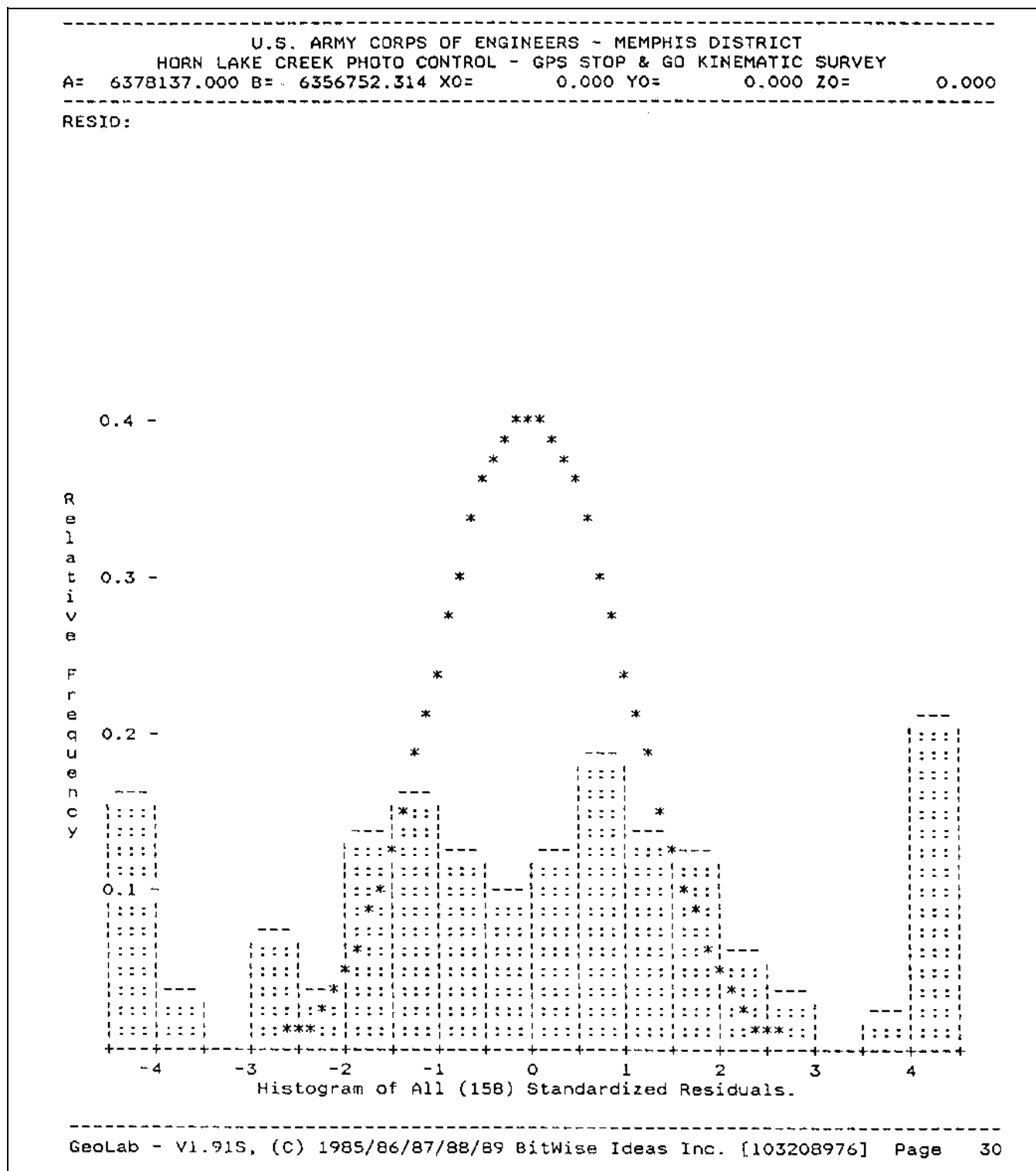


Figure E-19. (Sheet 2 of 6)

```
-----  
                U.S. ARMY CORPS OF ENGINEERS - MEMPHIS DISTRICT  
            HORN LAKE CREEK PHOTO CONTROL - GPS STOP & GO KINEMATIC SURVEY  
A= 6378137.000 B= 6356752.314 X0=      0.000 Y0=      0.000 Z0=      0.000  
-----
```

RESID:

```
-----  
                S T A T I S T I C S      S U M M A R Y  
-----
```

Residual Critical Value Type	Tau Max
Residual Critical Value	3.6597
Convergence Criterion	0.001000
Final Iteration Counter Value	2
Confidence Level Used	95.0000
Number of Flagged Residuals	35
Estimated Variance Factor	13.2102
Number of Degrees of Freedom	89

```
-----  
                Chi-Square Test on the Variance Factor:  
                1.0050e+001 < 1.0000 < 1.8146e+001 ?  
-----
```

```
                !!!!!!!!!!!!!!!!!!!!!!! THE TEST FAILS !!!!!!!!!!!!!!!!!!!!!!!  
-----
```

RESID successfully completed.

Figure E-19. (Sheet 3 of 6)

1 Aug 96

```

-----
                U.S. ARMY CORPS OF ENGINEERS - MEMPHIS DISTRICT
            HORN LAKE CREEK PHOTO CONTROL - GPS STOP & GO KINEMATIC SURVEY
A= 6378137.000 B= 6356752.314 X0= 0.000 Y0= 0.000 Z0= 0.000
-----

```

ELLIPSE:

2-D AND 1-D STATION CONFIDENCE REGIONS ( 95.000 %):

IDENT.	MAJOR SEMI-AXIS	MINOR SEMI-AXIS	AZ(MAJ)	VERTICAL
4098	0.0040	0.0034	145.07	0.0099
4097	0.0133	0.0117	103.56	0.0276
5211	0.0103	0.0093	74.85	0.0199
5221	0.0274	0.0165	4.04	0.0642
5208	0.0423	0.0161	1.90	0.0710
5213	0.0094	0.0070	113.66	0.0140
2099	0.0228	0.0152	23.58	0.0481
5212	0.0272	0.0101	9.26	0.1179
5214	0.0167	0.0093	153.80	0.0486
5215	0.0090	0.0065	160.93	0.0259
5216	0.0066	0.0042	157.23	0.0235
5217	0.0182	0.0128	143.89	0.0411
5219	0.0200	0.0123	11.25	0.0385
5220	0.0170	0.0114	17.47	0.0353
7212	0.0121	0.0085	24.83	0.0262
5222	0.0082	0.0068	87.45	0.0166
5224	0.0180	0.0146	94.13	0.0385
5223	0.0093	0.0075	101.03	0.0210
5209	0.0089	0.0072	66.56	0.0136
5218	0.0147	0.0116	127.81	0.0241
5206	0.0139	0.0103	142.23	0.0315
5201	0.0150	0.0098	28.21	0.0311
5203	0.0137	0.0052	9.10	0.0646

Figure E-19. (Sheet 4 of 6)

U.S. ARMY CORPS OF ENGINEERS - MEMPHIS DISTRICT							
HORN LAKE CREEK PHOTO CONTROL - GPS STOP & GO KINEMATIC SURVEY							
A= 6378137.000		B= 6356752.314		X0= 0.000	Y0= 0.000	Z0= 0.000	
ELLIPSE:							
2-D AND 1-D RELATIVE STATION CONFIDENCE REGIONS ( 95.000 %):							
FROM	TO	MAJ.SEMI	MIN.SEMI	AZ(MAJ)	VERTICAL	SPATIAL DIST.	PRECISION
4098	4097	0.0131	0.0116	99.26	0.0271	366.6987	35.831 PPM
4098	5211	0.0098	0.0085	72.49	0.0174	2912.1468	3.381 PPM
4098	5221	0.0276	0.0167	3.85	0.0647	4613.1075	5.981 PPM
4098	5208	0.0422	0.0159	1.94	0.0707	2733.5206	15.445 PPM
4098	2099	0.0226	0.0149	23.87	0.0473	2209.5901	10.210 PPM
4098	5212	0.0270	0.0096	9.41	0.1176	2184.0857	12.372 PPM
4098	5214	0.0167	0.0093	153.65	0.0477	1476.6292	11.304 PPM
4098	5215	0.0093	0.0068	159.03	0.0256	801.5979	11.644 PPM
4098	5216	0.0070	0.0045	155.32	0.0219	542.8330	12.936 PPM
4098	5217	0.0180	0.0127	143.88	0.0407	661.3897	27.271 PPM
4098	5219	0.0200	0.0124	11.12	0.0388	2891.5879	6.933 PPM
4098	5220	0.0167	0.0109	18.11	0.0342	2566.7336	6.506 PPM
4098	7212	0.0116	0.0077	26.24	0.0244	2016.1771	5.776 PPM
4098	5222	0.0083	0.0070	88.31	0.0170	4430.4238	1.873 PPM
4098	5224	0.0180	0.0146	94.14	0.0385	5477.8695	3.280 PPM
4098	5213	0.0090	0.0067	113.86	0.0135	1463.0782	6.169 PPM
4098	5223	0.0095	0.0077	101.99	0.0215	5905.2073	1.611 PPM
4098	5209	0.0093	0.0078	67.94	0.0157	2151.8403	4.314 PPM
4098	5218	0.0143	0.0112	127.13	0.0226	1886.0714	7.572 PPM
4098	5206	0.0141	0.0105	142.28	0.0321	3467.4980	4.065 PPM
4098	5203	0.0136	0.0049	9.29	0.0644	5895.6951	2.311 PPM
4098	5201	0.0151	0.0100	27.97	0.0315	6501.9487	2.326 PPM
4098	3095	0.0040	0.0034	145.07	0.0099	6444.1920	0.620 PPM
4097	3095	0.0133	0.0117	103.56	0.0276	6257.9643	2.119 PPM
3095	5211	0.0103	0.0093	74.85	0.0199	3561.8789	2.885 PPM
3095	5208	0.0423	0.0161	1.90	0.0710	3860.2203	10.956 PPM
3095	5221	0.0274	0.0165	4.04	0.0642	2049.7523	13.373 PPM
3095	5213	0.0094	0.0070	113.66	0.0140	5423.8102	1.726 PPM
3095	2099	0.0228	0.0152	23.58	0.0481	6228.3617	3.653 PPM
3095	5212	0.0272	0.0101	9.26	0.1179	4670.3272	5.823 PPM
3095	5214	0.0167	0.0093	153.80	0.0486	5123.7294	3.250 PPM
3095	5215	0.0090	0.0065	160.93	0.0259	6286.9790	1.435 PPM
3095	5216	0.0066	0.0042	157.23	0.0235	6093.8876	1.090 PPM
3095	5217	0.0182	0.0128	143.89	0.0411	6657.5119	2.732 PPM
3095	5219	0.0200	0.0123	11.25	0.0385	3553.0171	5.620 PPM
3095	5220	0.0170	0.0114	17.47	0.0353	4130.2059	4.118 PPM
3095	7212	0.0121	0.0085	24.83	0.0262	5986.6129	2.026 PPM
3095	5222	0.0082	0.0068	87.45	0.0166	2948.5833	2.778 PPM
3095	5224	0.0180	0.0146	94.13	0.0385	2553.4896	7.035 PPM
3095	5223	0.0093	0.0075	101.03	0.0210	1964.4001	4.759 PPM
3095	5209	0.0089	0.0072	66.56	0.0136	4497.4308	1.974 PPM
3095	5218	0.0147	0.0116	127.81	0.0241	4558.2390	3.215 PPM
3095	5206	0.0139	0.0103	142.23	0.0315	4289.4539	3.234 PPM
3095	5201	0.0150	0.0098	28.21	0.0311	7706.4128	1.948 PPM
GeoLab - V1.91S, (C) 1985/86/87/88/89 BitWise Ideas Inc. [103208976] Page 34							

Figure E-19. (Sheet 5 of 6)

1 Aug 96

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-----
                U.S. ARMY CORPS OF ENGINEERS - MEMPHIS DISTRICT
              HORN LAKE CREEK PHOTO CONTROL - GPS STOP & GO KINEMATIC SURVEY
A= 6378137.000 B= 6356752.314 X0=      0.000 Y0=      0.000 Z0=      0.000
-----

```

ELLIPSE:

2-D AND 1-D RELATIVE STATION CONFIDENCE REGIONS ( 95.000 %):

```

-----
FROM      TO      MAJ.SEMI MIN.SEMI AZ(MAJ) VERTICAL SPATIAL DIST.  PRECISION
-----
3095      5203      0.0137  0.0052   9.10   0.0646   6734.0532  2.040 PPM
-----

```

ELLIPSE successfully completed.  
10:06:34 - Tuesday, October 09, 1990

Figure E-19. (Sheet 6 of 6)



```

-----
                U.S. ARMY CORPS OF ENGINEERS - MEMPHIS DISTRICT
                HORN LAKE CREEK PHOTO CONTROL - GPS STOP & GO KINEMATIC SURVEY
A= 6378137.000 B= 6356752.314 X0=      0.000 Y0=      0.000 Z0=      0.000
-----
PREPARE:

10:29:30 - Tuesday, October 09, 1990

Input from:  <GPS7K_3.iob>
Output to:   <GPS7K_3.LST>

PREPARE successfully completed.

-----
GeoLab - V1.91S, (C) 1985/86/87/88/89 BitWise Ideas Inc. [103208976] Page    0

-----
                U.S. ARMY CORPS OF ENGINEERS - MEMPHIS DISTRICT
                HORN LAKE CREEK PHOTO CONTROL - GPS STOP & GO KINEMATIC SURVEY
A= 6378137.000 B= 6356752.314 X0=      0.000 Y0=      0.000 Z0=      0.000
-----
GETUP:

-----

```

PARAMETERS		OBSERVATIONS	
Description	Number	Description	Number
All Stations	24	Directions	0
Fixed Stations	1	Distances	0
Free 3-D Stations	23	Azimuths	0
Free 2-D Stations	0	Vertical Angles	0
Free 1-D Stations	0	Zenithal Angles	0
Coord. Parameters	69	Angles	0
Astro. Latitudes	0	Heights	3
Astro. Longitudes	0	Height Differences	0
Geoid Records	0	Auxiliary Params.	0
All Aux. Pars.	0	2-D Coords.	0
Direction Pars.	0	2-D Coord. Diffs.	0
Scale Parameters	0	3-D Coords.	0
Constant Pars.	0	3-D Coord. Diffs.	156
Rotation Pars.	0		
Translation Pars.	0		
	-----		-----
Total Parameters	69	Total Observations	159
		Degrees of Freedom = 90	

```

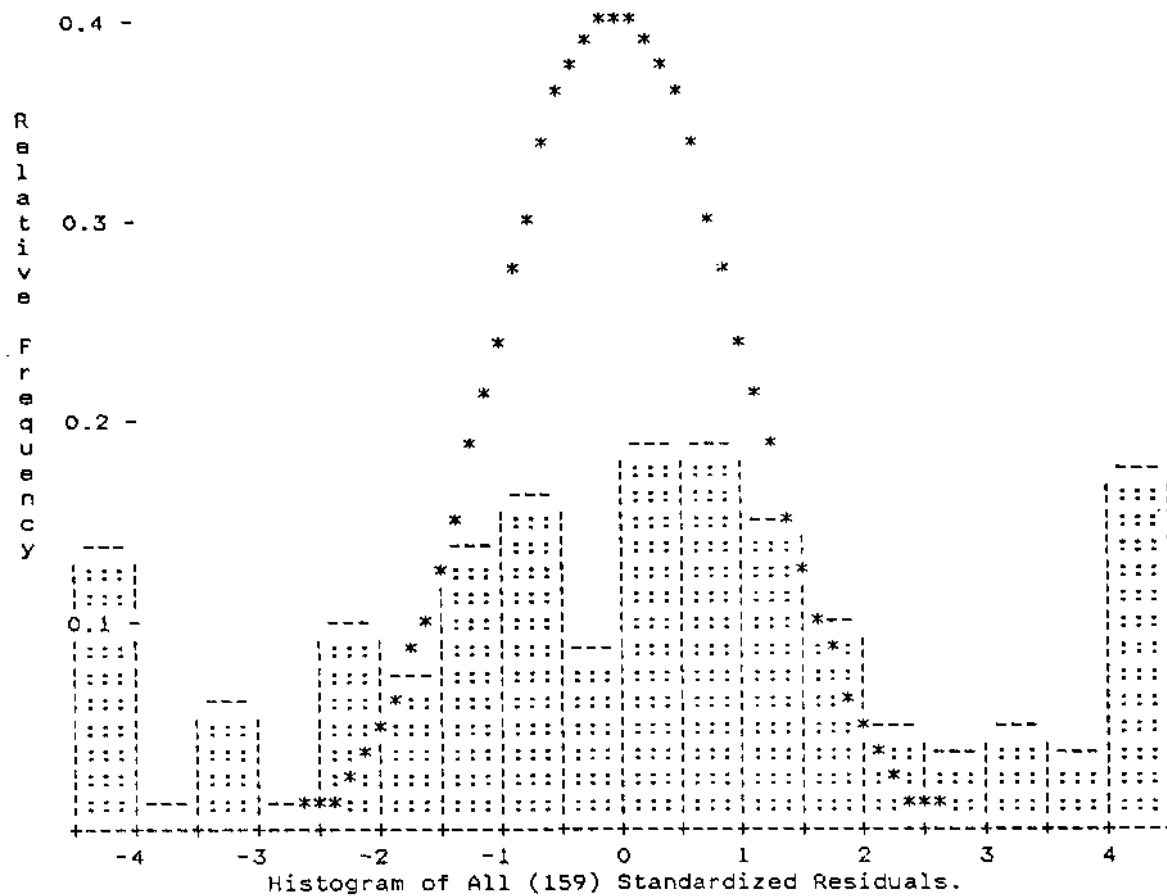
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GeoLab - V1.91S, (C) 1985/86/87/88/89 BitWise Ideas Inc. [103208976] Page    1
-----

```

Figure E-20. Fully constrained vertical adjustment (Sheet 1 of 6)

-----  
U.S. ARMY CORPS OF ENGINEERS - MEMPHIS DISTRICT  
HORN LAKE CREEK PHOTO CONTROL - GPS STOP & GO KINEMATIC SURVEY  
A= 6378137.000 B= 6356752.314 X0= 0.000 Y0= 0.000 Z0= 0.000  
-----

RESID:



-----  
GeoLab - V1.91S, (C) 1985/86/87/88/89 BitWise Ideas Inc. [103208976] Page 30  
-----

Figure E-20. (Sheet 2 of 6)

```

-----
                U.S. ARMY CORPS OF ENGINEERS - MEMPHIS DISTRICT
            HORN LAKE CREEK PHOTO CONTROL - GPS STOP & GO KINEMATIC SURVEY
A= 6378137.000 B= 6356752.314 X0=      0.000 Y0=      0.000 Z0=      0.000
-----

```

RESID:

```

-----
                S T A T I S T I C S      S U M M A R Y
-----

```

Residual Critical Value Type	Tau Max
Residual Critical Value	3.6624
Convergence Criterion	0.001000
Final Iteration Counter Value	2
Confidence Level Used	95.0000
Number of Flagged Residuals	30
Estimated Variance Factor	9.3212
Number of Degrees of Freedom	90

Chi-Square Test on the Variance Factor:

7.1012e+000 < 1.0000 < 1.2779e+001 ?

!!!!!!!!!!!!!!!!!!!!!!!!!!!! THE TEST FAILS !!!!!!!!!!!!!!!!!!!!!!!!!!!!!

RESID successfully completed.

Figure E-20. (Sheet 3 of 6)

1 Aug 96

-----  
 U.S. ARMY CORPS OF ENGINEERS - MEMPHIS DISTRICT  
 HORN LAKE CREEK PHOTO CONTROL - GPS STOP & GO KINEMATIC SURVEY  
 A= 6378137.000 B= 6356752.314 X0= 0.000 Y0= 0.000 Z0= 0.000  
 -----

ELLIPSE:

2-D AND 1-D STATION CONFIDENCE REGIONS ( 95.000 %):

IDENT.	MAJOR SEMI-AXIS	MINOR SEMI-AXIS	AZ(MAJ)	VERTICAL
4098	0.0037	0.0030	144.69	0.0080
4097	0.0112	0.0099	104.44	0.0232
5211	0.0087	0.0079	76.37	0.0166
5221	0.0230	0.0138	4.04	0.0539
5208	0.0355	0.0136	1.89	0.0596
5213	0.0079	0.0060	114.13	0.0117
2099	0.0129	0.0125	134.78	0.0006
5212	0.0229	0.0086	9.20	0.0990
5214	0.0140	0.0078	153.78	0.0407
5215	0.0076	0.0055	160.87	0.0217
5216	0.0056	0.0035	157.13	0.0196
5217	0.0153	0.0108	143.89	0.0345
5219	0.0168	0.0103	11.22	0.0323
5220	0.0098	0.0087	129.60	0.0006
7212	0.0103	0.0073	24.26	0.0219
5222	0.0069	0.0058	87.89	0.0139
5224	0.0151	0.0123	94.25	0.0323
5223	0.0079	0.0063	101.29	0.0176
5209	0.0075	0.0060	66.59	0.0114
5218	0.0124	0.0098	128.06	0.0201
5206	0.0117	0.0086	142.23	0.0264
5201	0.0090	0.0082	31.01	0.0006
5203	0.0116	0.0044	8.97	0.0542

Figure E-20. (Sheet 4 of 6)

1 Aug 96

U.S. ARMY CORPS OF ENGINEERS - MEMPHIS DISTRICT									
HORN LAKE CREEK PHOTO CONTROL - GPS STOP & GO KINEMATIC SURVEY									
A=	6378137.000	B=	6356752.314	X0=	0.000	Y0=	0.000	Z0=	0.000
-----									
ELLIPSE:									
-----									
2-D AND 1-D RELATIVE STATION CONFIDENCE REGIONS ( 95.000 %):									
-----									
FROM	TO	MAJ.SEMI	MIN.SEMI	AZ(MAJ)	VERTICAL	SPATIAL DIST.	PRECISION		
-----									
4098	4097	0.0110	0.0098	99.29	0.0228	366.6974	30.134 PPM		
4098	5211	0.0083	0.0072	72.51	0.0146	2912.1465	2.841 PPM		
4098	5221	0.0232	0.0141	3.77	0.0543	4613.1048	5.031 PPM		
4098	5208	0.0355	0.0133	1.94	0.0594	2733.5198	12.975 PPM		
4098	2099	0.0127	0.0123	153.61	0.0080	2209.5932	5.731 PPM		
4098	5212	0.0227	0.0081	9.40	0.0988	2184.0853	10.393 PPM		
4098	5214	0.0141	0.0078	153.61	0.0401	1476.6279	9.515 PPM		
4098	5215	0.0079	0.0058	158.62	0.0215	801.5977	9.882 PPM		
4098	5216	0.0060	0.0039	154.93	0.0184	542.8319	11.054 PPM		
4098	5217	0.0152	0.0106	143.88	0.0342	661.3901	22.922 PPM		
4098	5219	0.0169	0.0105	11.04	0.0326	2891.5859	5.832 PPM		
4098	5220	0.0093	0.0084	140.18	0.0080	2566.7382	3.642 PPM		
4098	7212	0.0098	0.0064	26.24	0.0205	2016.1771	4.852 PPM		
4098	5222	0.0070	0.0059	89.11	0.0142	4430.4221	1.583 PPM		
4098	5224	0.0151	0.0123	94.26	0.0323	5477.8680	2.758 PPM		
4098	5213	0.0076	0.0056	113.97	0.0113	1463.0774	5.189 PPM		
4098	5223	0.0080	0.0065	102.62	0.0180	5905.2054	1.361 PPM		
4098	5209	0.0079	0.0067	68.68	0.0131	2151.8379	3.652 PPM		
4098	5218	0.0120	0.0094	127.14	0.0190	1886.0709	6.362 PPM		
4098	5206	0.0119	0.0088	142.30	0.0269	3467.4963	3.430 PPM		
4098	5203	0.0115	0.0041	9.24	0.0541	5895.6947	1.943 PPM		
4098	5201	0.0092	0.0085	23.95	0.0080	6501.9478	1.419 PPM		
4098	3095	0.0037	0.0030	144.69	0.0080	6444.1888	0.579 PPM		
4097	3095	0.0112	0.0099	104.44	0.0232	6257.9628	1.785 PPM		
3095	5211	0.0087	0.0079	76.37	0.0166	3561.8760	2.441 PPM		
3095	5208	0.0355	0.0136	1.89	0.0596	3860.2179	9.207 PPM		
3095	5221	0.0230	0.0138	4.04	0.0539	2049.7519	11.234 PPM		
3095	5213	0.0079	0.0060	114.13	0.0117	5423.8080	1.464 PPM		
3095	2099	0.0129	0.0125	134.78	0.0006	6228.3515	2.073 PPM		
3095	5212	0.0229	0.0086	9.20	0.0990	4670.3246	4.898 PPM		
3095	5214	0.0140	0.0078	153.78	0.0407	5123.7278	2.733 PPM		
3095	5215	0.0076	0.0055	160.87	0.0217	6286.9777	1.207 PPM		
3095	5216	0.0056	0.0035	157.13	0.0196	6093.8863	0.919 PPM		
3095	5217	0.0153	0.0108	143.89	0.0345	6657.5098	2.301 PPM		
3095	5219	0.0168	0.0103	11.22	0.0323	3553.0159	4.723 PPM		
3095	5220	0.0098	0.0087	129.60	0.0006	4130.1976	2.364 PPM		
3095	7212	0.0103	0.0073	24.26	0.0219	5986.6100	1.714 PPM		
3095	5222	0.0069	0.0058	87.89	0.0139	2948.5823	2.341 PPM		
3095	5224	0.0151	0.0123	94.25	0.0323	2553.4888	5.915 PPM		
3095	5223	0.0079	0.0063	101.29	0.0176	1964.3997	4.007 PPM		
3095	5209	0.0075	0.0060	66.59	0.0114	4497.4302	1.659 PPM		
3095	5218	0.0124	0.0098	128.06	0.0201	4558.2364	2.717 PPM		
3095	5206	0.0117	0.0086	142.23	0.0264	4289.4530	2.719 PPM		
3095	5201	0.0090	0.0082	31.01	0.0006	7706.4127	1.165 PPM		

GeoLab - V1.91S, (C) 1985/86/87/88/89 BitWise Ideas Inc. [103208976] Page 34

Figure E-20. (Sheet 5 of 6)

1 Aug 96

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                U.S. ARMY CORPS OF ENGINEERS - MEMPHIS DISTRICT
            HORN LAKE CREEK PHOTO CONTROL - GPS STOP & GO KINEMATIC SURVEY
A= 6378137.000 B= 6356752.314 X0=      0.000 Y0=      0.000 Z0=      0.000
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ELLIPSE:

2-D AND 1-D RELATIVE STATION CONFIDENCE REGIONS ( 95.000 %):

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-----
FROM      TO      MAJ.SEMI MIN.SEMI AZ(MAJ) VERTICAL SPATIAL DIST.    PRECISION
-----
3095      5203      0.0116  0.0044  8.97  0:0542  6734.0520  1.718 PPM
-----

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ELLIPSE successfully completed.  
 10:36:17 ~ Tuesday, October 09, 1990

Figure E-20. (Sheet 6 of 6)

(2) 5 hr to set temporary photo control points and obtain visibility information at each site, 2-man crew.

(3) 2 hr to install two Type A monuments, 3-man crew.

(4) 5 hr to perform necessary Third-Order traverse and leveling, 4-man crew.

(5) 15 hr to collect static and stop-and-go GPS data, 5-man crew.

*b.* A conservative estimate of the field operations required by a 4-man party to 3D-position the 23 photo control points using conventional terrestrial methods was approximately 70 hr. The accuracies obtained from the final horizontal adjustment of the GPS data exceeded the requirements of the project. The accuracies obtained from the final vertical adjustment of the GPS data met the requirements.